

# PUBLIC ROADS

A JOURNAL OF HIGHWAY RESEARCH

FEDERAL WORKS AGENCY  
PUBLIC ROADS ADMINISTRATION

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VOL. 23, NO. 5



JULY-AUGUST-SEPTEMBER 1942



ENGINEERS MEASURE THE STRENGTH OF A MONOLITHIC BRICK TEST SLAB

# PUBLIC ROADS

►►► *A Journal of  
Highway Research*

*issued by the*

## FEDERAL WORKS AGENCY PUBLIC ROADS ADMINISTRATION

D. M. BEACH, *Editor*

Volume 23, No. 5

July-August-September 1942

*The reports of research published in this magazine are necessarily qualified by the conditions of the tests from which the data are obtained. Whenever it is deemed possible to do so, generalizations are drawn from the results of the tests; and, unless this is done, the conclusions formulated must be considered as specifically pertinent only to described conditions.*

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# TESTS OF MONOLITHIC BRICK PAVEMENT SLABS

BY THE DIVISION OF TESTS, PUBLIC ROADS ADMINISTRATION

Reported by W. F. KELLERMANN, Materials Engineer

IN the construction of brick pavements with concrete bases it has been customary during recent years to lay the brick in a bedding course of sand, or a mixture of sand and bitumen, that has been spread on the hardened concrete base, and to fill the joints between the brick with bitumen. With this type of construction, known as the two-course type, the brick wearing course could not be expected to add appreciably to the strength of the concrete base.

A number of years ago monolithic brick pavements were constructed by laying the brick in the fresh concrete of the base, rolling with a light roller, and filling the joints with cement grout. At that time the need for expansion joints was not fully appreciated and much trouble was experienced with blow-ups caused by expansion. Also the riding qualities of monolithic brick pavements did not compare favorably with the riding qualities of other high-type pavements. As a result, the monolithic type fell into disuse and the two-course type was used exclusively in brick pavement construction.

With the introduction of surface vibration as a means of compacting the concrete in pavement slabs, new possibilities were presented for the construction of monolithic brick pavements. Consequently, several monolithic brick pavements have been built in which the brick have been embedded in the concrete base by means of high-frequency surface vibrators, rather than by rollers.

The procedure is first to build the usual concrete base using a finishing machine equipped with surface vibrators. The brick are then immediately placed on the fresh concrete and vibrated with a machine designed especially for the purpose. This machine is built along the same general lines as the conventional finishing machine except that in place of the conventional screeds there is a special screed consisting of a series of steel channels. These channels are vibrated as they ride over the brick, with the result that the brick are brought to proper elevation, at the same time being forced down about  $\frac{1}{8}$  inch into the concrete. A thin sand-cement grout is then squeegeed over the brick surface, filling the joints. After the grout has taken its initial set, a second application of somewhat stiffer grout is applied.

Some years ago monolithic pavements were constructed by placing the brick in the fresh concrete of the base and filling the joints with mortar. This type of pavement was discarded in favor of the conventional two-course brick pavement (1) because of trouble experienced with blow-ups which resulted from the absence of adequate expansion joints and (2) because the riding qualities were not as good as those of other high type pavements.

This report describes a recent development in the technique of constructing monolithic brick pavements and discusses the results of a series of tests made to determine the quality of pavement slabs so constructed as compared with the quality of pavement slabs constructed entirely of concrete.

The results of these tests have indicated that monolithic brick slabs, when tested with the concrete in tension, developed higher flexural strengths than comparable slabs of plain concrete. However, when the brick surface was placed in tension, the monolithic brick slabs developed much lower strengths than comparable slabs of plain concrete. In the latter case the angle at which the brick courses were placed with respect to the loading knife edge of the testing machine had an effect upon the results obtained.

In tests made with the brick surface in tension, the bond between the concrete and the brick was not affected by 100 applications of load equal to 50 percent of the ultimate load.

A final mortar finish over the entire surface is obtained by use of a burlap drag. It is claimed that this type of construction eliminates the objectionable features of the old-type monolithic pavement and that pavements constructed by this method are superior in many respects to the two-course type.

## TWO SERIES OF TESTS MADE

In order to determine the flexural strengths of monolithic brick slabs constructed in this manner as compared with those of plain concrete compacted by surface vibration, two series of laboratory tests (series A and series B) were initiated in which the following variables were studied:

### SERIES A

1. Effect of total depth of slab.
2. Effect of testing the top surface of the slab, as cast, in tension as compared with the bottom surface.
3. Effect of angle of brick courses with respect to the longitudinal axis of slab.
4. Effect of repeated loadings.

### SERIES B

1. Effect of testing the top surface of the slab in tension as compared with the bottom surface.
2. Effect of angle of brick courses with respect to longitudinal axis of slab.
3. Effect of richness of concrete mix.

The brick used in these tests were the conventional 3- by 4- by  $8\frac{1}{2}$ -inch vertical fiber lug brick normally used in pavement construction. Grout used as a filler was proportioned 1:2 by dry loose volume with sufficient water to give the desired consistency. Tables 1 and 2 give the physical properties of the aggregate while the mix data for the concrete and the grout are given in tables 3 and 4, respectively. Different cements were used in the two series. Results of strength tests on the two cements are as follows:

	<i>Tensile strength, lb. per sq. in.</i>
Series A:	
At 7 days	405
At 28 days	485
Series B:	
At 7 days	380
At 28 days	435

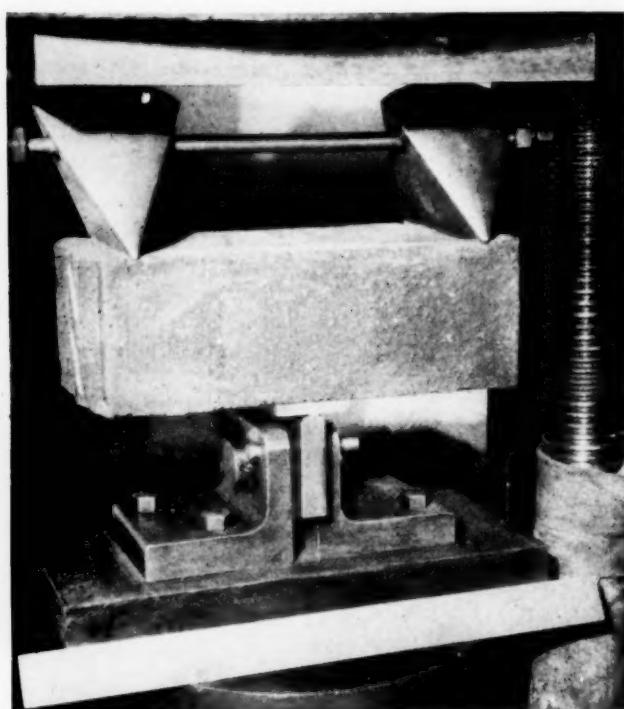


FIGURE 1.—SET-UP USED IN DETERMINING FLEXURAL STRENGTH OF PAVING BRICK.

TABLE 1.—Physical properties of aggregates for monolithic brick pavement slabs

Aggregates	Character of aggregate	Bulk specific gravity	Weight per cubic foot		Voids, dry rodded	Absorption	Los Angeles abrasion loss <sup>1</sup>
			Solid	Dry rodded			
For concrete:							
Sand, series A	Quartz	2.59	162	106	35	1.00	—
Sand, series B	do	2.57	160	105	34	1.50	—
Gravel	do	2.58	161	107	34	.99	28.0
For grout: Sand	do	2.60	162	100	244	1.00	—

<sup>1</sup> Grading A used.

<sup>2</sup> Dry loose.

TABLE 2.—Grading of aggregates for monolithic brick pavement slabs

Sieve size	Mortar sand	Percentage retained		
		Concrete sand		Gravel
		Series A	Series B	
1½-inch	0	0	0	0
1-inch	0	0	0	38
¾-inch	0	0	0	57
½-inch	0	0	0	86
No. 4	0	1	1	100
No. 8	0	13	16	100
No. 16	0	25	31	100
No. 30	17	42	54	100
No. 50	65	74	83	100
No. 100	94	95	96	100
Fineness modulus	1.76	2.50	2.81	7.43

Results of tests made on the brick are given in table 5. The set-up for determining the flexural strength of the brick is shown in figure 1.

In this report the term "pavement concrete" is used to designate concrete containing 6 sacks of cement per cubic yard, which is a class commonly used in pavements. The term "base concrete" is used to designate concrete containing 4½ sacks of cement per cubic yard,

which is a class frequently used in the construction of bases.

Pavement concrete was used in series A, with sufficient water to produce a slump of approximately 1 inch.

Test specimens consisted of slabs approximately 26 inches wide by 60 inches long and 7, 8, and 10 inches in depth. Both plain concrete and monolithic brick slabs were cast for each thickness.

TABLE 3.—Data on concrete mixes for monolithic brick pavement slabs

Proportions by weight, pounds	W/C by volume	W <sub>c</sub> by volume (%)	b/b <sub>0</sub> (2)	Mortar voids ratio	Sand in total aggregate, by weight	Cement factor	Slump
Series A: 94 : 173 : 367	0.65	0.145	0.77	1.87	Percent 32	Sacks per cu. yd. 6.0	Inches 1.0
Series B:							
94 : 184 : 356	.65	.145	.74	2.02	34	6.0	.75
94 : 253 : 451	.84	.148	.74	2.04	36	4.75	.75

<sup>1</sup> Water per unit volume of concrete.

<sup>2</sup> Apparent volume of coarse aggregate per unit volume of concrete, dry rodded basis.

TABLE 4.—Data on grout used in monolithic brick pavement slabs, series A and B

Proportions		W/C by volume	
By dry loose volume	By weight	First grout	Second grout
1:2	Pounds 94:180	1.01	0.86

TABLE 5.—Physical properties of paving brick used in monolithic brick pavement slabs

Test	Average <sup>1</sup>	Maximum	Minimum
Absorption	percent	4.54	5.67
Specific gravity	2.36	2.39	2.32
Standard rattle	percent	13.6	—
Modulus of rupture—pounds per square inch	2,250	2,540	1,980
Crushing strength	do	18,380	21,690
		12,830	

<sup>1</sup> Each result is the average for 10 tests except where otherwise noted.

<sup>2</sup> One test.

The plain concrete slabs were constructed in the following manner: The form was filled and an excess of concrete placed on top in order to allow for subsidence. Two complete passes were then made over the entire surface with a platform vibrator operating at a frequency of 7,000 pulsations per minute. The slab was then struck off with a wooden screed. In the case of the monolithic brick slabs the form was filled to within 2½ inches of the top and the concrete vibrated in the same manner. The brick were then set in the fresh concrete and vibrated in the same manner as for the plain concrete slabs (see fig. 2). This procedure resulted in twice as much vibration of the concrete in the monolithic as in the plain concrete slabs. It was adopted because it followed actual practice for the two different types of construction in the field.

After vibrating, a 1:2 mortar grout sufficiently thin to penetrate the joints was squeegeed over the surface. As this grout stiffened it also subsided, leaving the joints low. A second and somewhat thicker grout was then applied to fill the joints. The final finish was with a

burlap drag, leaving a thin mortar top over the entire surface.

All slabs were kept under wet burlap for 2 days, after which they were removed from the molds and cured in a fog room at 70° F. until tested at 29 days. In the monolithic brick slabs, three different arrangements of the brick were used. In one case the brick were laid with the courses parallel to the longitudinal axis of the slab. In the second case they were laid transversely to the longitudinal axis of the slab; while in the third case they were laid at an angle of 45°.

One group of slabs was tested with the bottom as cast in tension; the other group was tested with the top or brick surface in tension. Those slabs tested with the bottom in tension were made in groups of three, of which one was a plain concrete slab and two were monolithic brick. When testing the concrete slab and one of the monolithic slabs, each slab was loaded to failure with one application of loading. This method of loading will be referred to as instantaneous loading. The other monolithic slab was given 100 applications of a load equivalent to one-half the ultimate load obtained in the test of the first monolithic slab. The specimen was then loaded to failure. This procedure was followed in order to determine whether 100 repetitions of 50 percent of the ultimate load would produce any separation of the brick surface from the concrete base. Table 6 gives a complete outline of the procedure together with the strength results. All slabs were tested in a portable testing machine, using a pair of calibrated, heat-treated steel beams as a load-measuring device. The cover illustration shows a set-up of the testing equipment. The specimens were tested as simple beams with the load applied at the center of a 54-inch span.

TABLE 6.—Summary of results of flexure tests on monolithic brick pavement slabs, series A<sup>1</sup>

Structure	Nominal depth of slab	Surface in tension <sup>2</sup>	Direction of knife edge of applied load <sup>3</sup>	Coarse aggregate broken	Flexural strength, <sup>4</sup> age 29 days	Strength ratio <sup>5</sup>
	Inches			Percent	Lb./sq. in.	Percent
Concrete	7	Bottom			55	730
Monolithic brick	7	do	Parallel	60	868	119
Do	7	do	do <sup>6</sup>	70	841	115
Concrete	8	do		60	741	100
Monolithic brick	8	do	Parallel	65	840	113
Do	8	do	do <sup>6</sup>	55	788	106
Concrete	10	do		50	728	100
Monolithic brick	10	do	Parallel	65	859	118
Do	10	do	do <sup>6</sup>	60	884	121
Concrete	7	Top		55	797	100
Monolithic brick	7	do	Perpendicular	65	636	80
Do	7	do	45° angle	70	535	67
Do	7	do	Parallel	45	341	43
Concrete	8	do		60	845	100
Monolithic brick	8	do	Perpendicular	50	724	86
Do	8	do	45° angle	50	508	60
Do	8	do	Parallel	40	367	43
Concrete	10	do		50	776	100
Monolithic brick	10	do	Perpendicular	60	790	102
Do	10	do	45° angle	40	588	76
Do	10	do	Parallel	55	443	57

<sup>1</sup> Specimens 26 inches wide by 60 inches long (approximately); tested with center loading on 54-inch span after 29 days of moist curing.

<sup>2</sup> In all cases where the slabs were tested with the bottom surface in tension, each result is the average of two tests. All other values are individual test results.

<sup>3</sup> With respect to the line of continuous brick courses.

<sup>4</sup> Flexural strength computed with the neutral axis assumed at the center of the slab.

<sup>5</sup> Plain concrete slabs in each case were taken as standard.

<sup>6</sup> Given 100 repetitions of a load equal to 50 percent of the ultimate load of the corresponding slab and then tested to failure.

#### RICHNESS OF CONCRETE MIXTURE A VARIABLE IN SERIES B

In series B the same procedure of casting, curing, and testing was employed as in series A, except that in the brick slabs that were tested with the top, as cast, in



FIGURE 2.—VIBRATOR USED FOR COMPACTING PLAIN CONCRETE AND MONOLITHIC BRICK SLABS.

tension, the brick were placed at an angle of 45° with respect to the longitudinal axis instead of in three different ways, as was done in series A. Also, all specimens were tested by instantaneous loading. Tests were made at 28 days instead of at 29 days as in series A.

One variable, not included in series A but studied in series B, was the effect of the richness of the concrete mix. In series A pavement concrete was used in all slabs, while in series B both pavement concrete and base concrete were used. This was done in order to compare the strength of monolithic brick slabs of base concrete directly with the strength of vibrated pavement concrete of the same total thickness. The effect of depth of slab was not studied in series B, all specimens being 8 inches thick.

Strength results are tabulated in table 6 and shown in graphical form in figure 3. In computing the values for modulus of rupture it was assumed that the neutral axis was at the center of the slab. It is realized that this may not be a correct assumption, particularly in the case of the monolithic brick slabs. However, for purposes of determining comparative load-carrying capacity, this procedure is justified, since comparisons are made between monolithic brick and plain concrete specimens of equal thickness. The same comparisons could be made with the total breaking loads.

Considering first the results plotted in figure 3 for those specimens tested with the bottom, as cast, in tension (brick surface in compression) the monolithic brick slabs gave higher values for computed modulii of rupture than did those of plain concrete. This was true for slabs of all thicknesses tested. The modulus of rupture of the monolithic brick specimens tested instantaneously averaged 17 percent higher than that of the plain concrete, while for the monolithic slabs given 100 repetitions of 50 percent loading the modulus of rupture averaged 14 percent higher than that of the plain concrete. It will be noted that in two out of three cases the monolithic brick slabs given 100 repetitions of 50 percent of the ultimate load and then loaded to failure gave slightly lower strengths than did

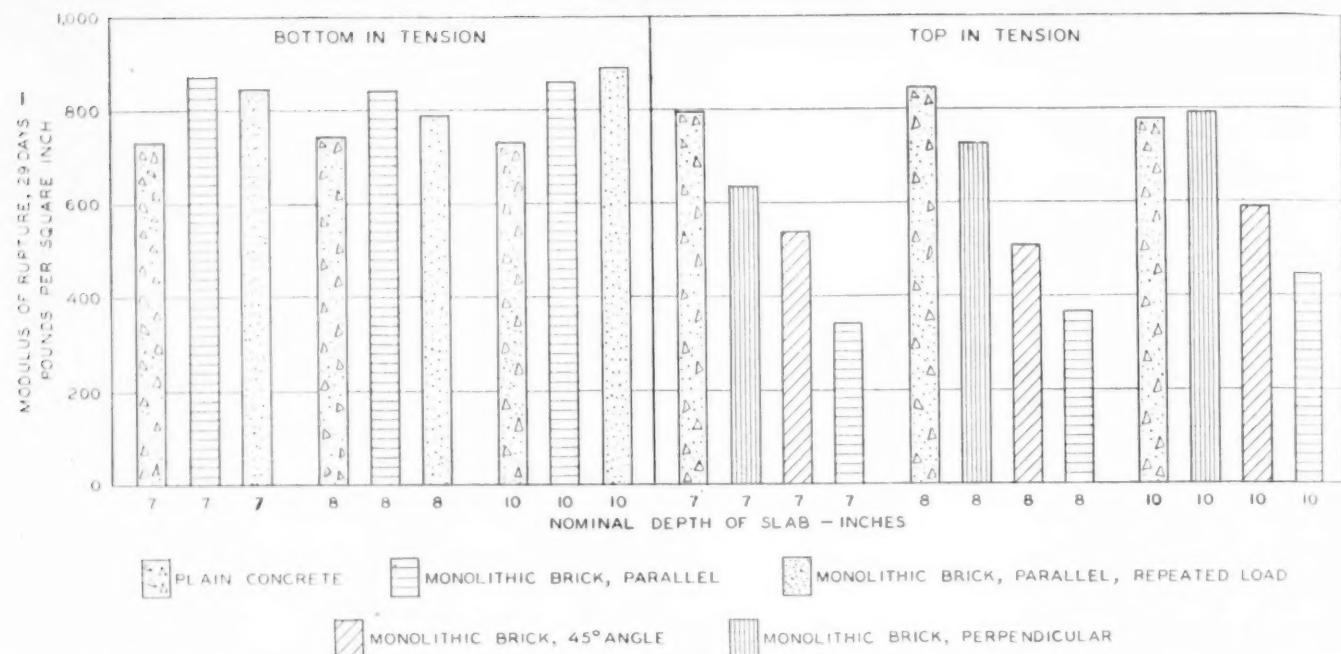


FIGURE 3.—RESULTS OF FLEXURE TESTS ON MONOLITHIC BRICK PAVEMENT SLABS, SERIES A, AT AGE OF 29 DAYS. "PARALLEL," "45° ANGLE," AND "PERPENDICULAR" REFER TO THE DIRECTION OF KNIFE EDGE OF APPLIED LOAD WITH RESPECT TO THE LINE OF CONTINUOUS BRICK COURSES. "REPEATED LOAD" INDICATES SLABS WERE GIVEN 100 REPETITIONS OF A LOAD EQUAL TO 50 PERCENT OF THE ULTIMATE LOAD OF THE CORRESPONDING SLAB AND THEN TESTED TO FAILURE.

the corresponding slabs tested with but one application of loading. It was anticipated that the repeated flexing of the slab might tend to destroy the bond between the brick surface and the concrete base but apparently this did not take place since the differences between the results obtained with the single loading and after repeated loading are within the limits of experimental error.

In all cases where the slabs were tested after repeated loadings, the percentage of brick broken at the section was less than in cases where they were tested with but one application of load. In the case of the 7-inch monolithic brick slabs tested by instantaneous loading the percentage of brick broken was 40 while for the companion slabs tested after repeated loading the percentage was 20. For the 8-inch slabs the differential was even greater whereas for the 10-inch slabs it was somewhat less.

Data for the slabs tested with the top surface, as cast, in tension are also plotted in figure 3. In this group no specimens were given repeated loadings. However, the brick were placed in three different directions so that there were four slabs in each group, one of plain concrete and three of monolithic brick. In each group the left bar represents the strength of the plain concrete while the second bar represents the strength of monolithic brick slabs in which the line of continuous courses of brick was perpendicular to the axis of the loading knife edge of the testing machine. In this case the maximum bending moment occurred at the center of the individual brick in three of the six brick courses in the cross section, while, due to the staggering of courses, the maximum bending moment occurred at the joint between brick and mortar in the other three cases. This is illustrated in figure 4-A, which shows a view of a fractured monolithic slab.

The third bar in the group gives the strength of monolithic brick slabs in which the direction of brick

courses was at an angle of 45° to the axis of the loading knife edge. In this test failure occurred at the joint between the mortar and the brick (see fig. 4-B). The fourth bar in each group gives the strength of monolithic brick slabs in which the courses of brick were parallel to the axis of the loading knife edge. As was to be expected from this procedure, failure occurred at the joint between the brick and the mortar as shown in figure 4-C.

#### STRENGTH AFFECTED BY DIRECTION OF BRICK COURSES

The direction of the courses of brick had a marked influence upon the strength. Of the three ways of placing the brick, the highest strengths were obtained by placing the line of brick courses perpendicular to the axis of the loading knife edge, while the lowest strengths were obtained by placing the line of brick courses parallel to the axis of the loading knife edge. Brick courses placed at an angle of 45° to the loading knife edge gave intermediate results. Because of the failure in bond between the brick and grout, the monolithic brick slabs tested with the brick in tension gave much lower results than did the plain concrete slabs, the only exception being the 10-inch monolithic slabs with the brick courses perpendicular to the axis of the loading knife edge, which gave results slightly higher than those obtained with the plain concrete.

In summary, it was found that monolithic brick slabs tested with the concrete surface in tension gave results about 17 percent higher than those for plain concrete of equal thickness. On the other hand, when tested with the brick surface in tension and with the courses of brick set parallel to the axis of the loading knife edge, monolithic brick averaged only about 50 percent of the strength of plain concrete. The direction of the brick courses in the slabs had a marked effect in this test. Those slabs in which the courses of brick were set

(Continued on page 97)

# VERTICAL CURVES FOR HIGHWAYS

BY THE DIVISION OF DESIGN, PUBLIC ROADS ADMINISTRATION

Reported by D. W. LOUTZENHEISER, Associate Highway Engineer

*Length of vertical curve.*—Any vertical curve on a highway should be as long as possible but the minimum length should be based on the sight distance requirements for the highway. The sight distances required are based on the assumed design speed for the highway. At any point on a highway the sight distance should be at least long enough that a vehicle traveling at the design speed can, upon sight of an object on the road, be brought to a stop before reaching it. Such sight distances are known as "nonpassing sight distances" and are not long enough on 2-lane roads for safe passing of vehicles in the same direction in the face of opposing traffic. Sight distances safe for passing on 2- and 3-lane roads are known as "passing sight distances." They rarely can be provided at all points, but they should be provided at frequent intervals.

The pamphlet "A Policy on Sight Distances for Highways," published by the American Association of State Highway Officials, presents a discussion of pertinent factors, calculations of and recommendations for minimum values of both nonpassing and passing sight distances for various highway design speeds. Tables 1 and 2 give the minimum lengths of vertical curve necessary for the various design speed-sight distance requirements. Reference is made to the sight distance policy for the numerical values of the various sight distances and formulas for calculation.

TABLE 1.—Minimum length of vertical curves to provide minimum nonpassing sight distance

A=algebraic difference of grades—percent	Length of vertical curve for assumed design speed of—				
	30 mi. per hr.	40 mi. per hr.	50 mi. per hr.	60 mi. per hr.	70 mi. per hr.
2	Stations 0	Stations 0	Stations 0.9	Stations 2.2	Stations 4.7
3	0	.6	2.2	4.6	7.4
4	.4	1.8	3.4	6.2	9.9
5	1.1	2.5	4.2	7.7	12.4
6	1.6	3.1	5.0	9.3	14.8
7	1.9	3.6	5.9	10.8	17.3
8	2.2	4.2	6.7	12.4	19.8
9	2.5	4.7	7.6	13.9	22.2
10	2.8	5.2	8.4	15.5	24.7
11	3.0	5.7	9.2	17.0	
12	3.3	6.2	10.1	18.6	
13	3.6	6.8	10.9	20.1	
14	3.8	7.3	11.8	21.7	
15	4.1	7.8	12.6	23.2	
16	4.4	8.3	13.5		
17	4.7	8.8	14.3		
18	4.9	9.3	15.1		
19	5.2	9.8	16.0		
20	5.5	10.4	16.8		

*Calculations of elevations on vertical curves.* Terms used are as follows (see fig. 1):

$g_1$  and  $g_2$ =grades, percent;

$A$ =algebraic difference of grades, percent;

$L$ =length of vertical curve, stations;

$t$ =distance from end of curve, feet; and

$d$ =vertical distance between grade line and vertical curve, feet.

The basic formulas are:

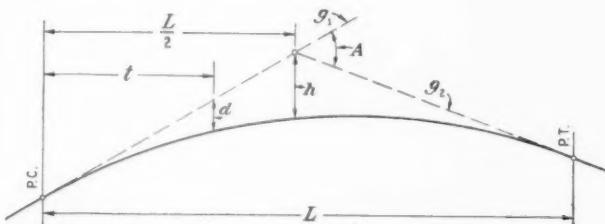


FIGURE 1.—CALCULATION OF ELEVATIONS ON VERTICAL CURVES.

TABLE 2.—Minimum length of vertical curves to provide minimum passing sight distance

A=algebraic difference of grades—percent	Length of vertical curve on 2-lane roads for assumed design speed of—					Length of vertical curve on 3-lane roads for assumed design speed of—		
	30 mi. per hr.	40 mi. per hr.	50 mi. per hr.	60 mi. per hr.	70 mi. per hr.	50 mi. per hr.	60 mi. per hr.	70 mi. per hr.
1.00						6.0	22.0	
1.25						0	29.2	
1.50						4.0	35.0	
1.75						7.4	40.9	
2.00						0	46.7	0
2.5						3.6	13.6	30.6
3.0						6.0	16.3	36.8
3.5						0	19.1	42.9
4.0						1.0	21.8	49.0
4.5						2.0	10.1	24.5
5.0						2.8	11.2	27.2
5.5						3.5	12.4	29.9
6.0						4.0	13.5	32.7
7						4.9	15.8	38.1
8						5.6	18.0	43.6
9						6.2	20.2	49.0
10						6.9	22.5	
11						7.6	24.8	
12						8.3	27.0	
13						9.0	29.2	
14						9.7	31.5	
15						10.4	33.8	
16						11.1	36.0	

$$g_1 - g_2 = A$$

$$h = \frac{AL}{8}$$

$$d = \frac{h^2}{2,500 L^2}$$

The value of  $d$  may be expressed as

$$d = \frac{A}{L} \times K$$

where

$$K = \frac{t^2}{20,000}$$

Table 3 gives values of  $\frac{A}{L}$ . Values given are for whole numbers of  $A$  and  $L$ ; for intermediate values interpolate or calculate  $\frac{A}{L}$  directly. Table 4 gives values of  $K$  for various values of  $t$ . Then  $d = \text{value of } \frac{A}{L} \text{ from table 3 times value of } K \text{ from table 4.}$

Figure 2 is a nomograph which permits a more rapid but less accurate solution for one of the above factors when the other three are known.

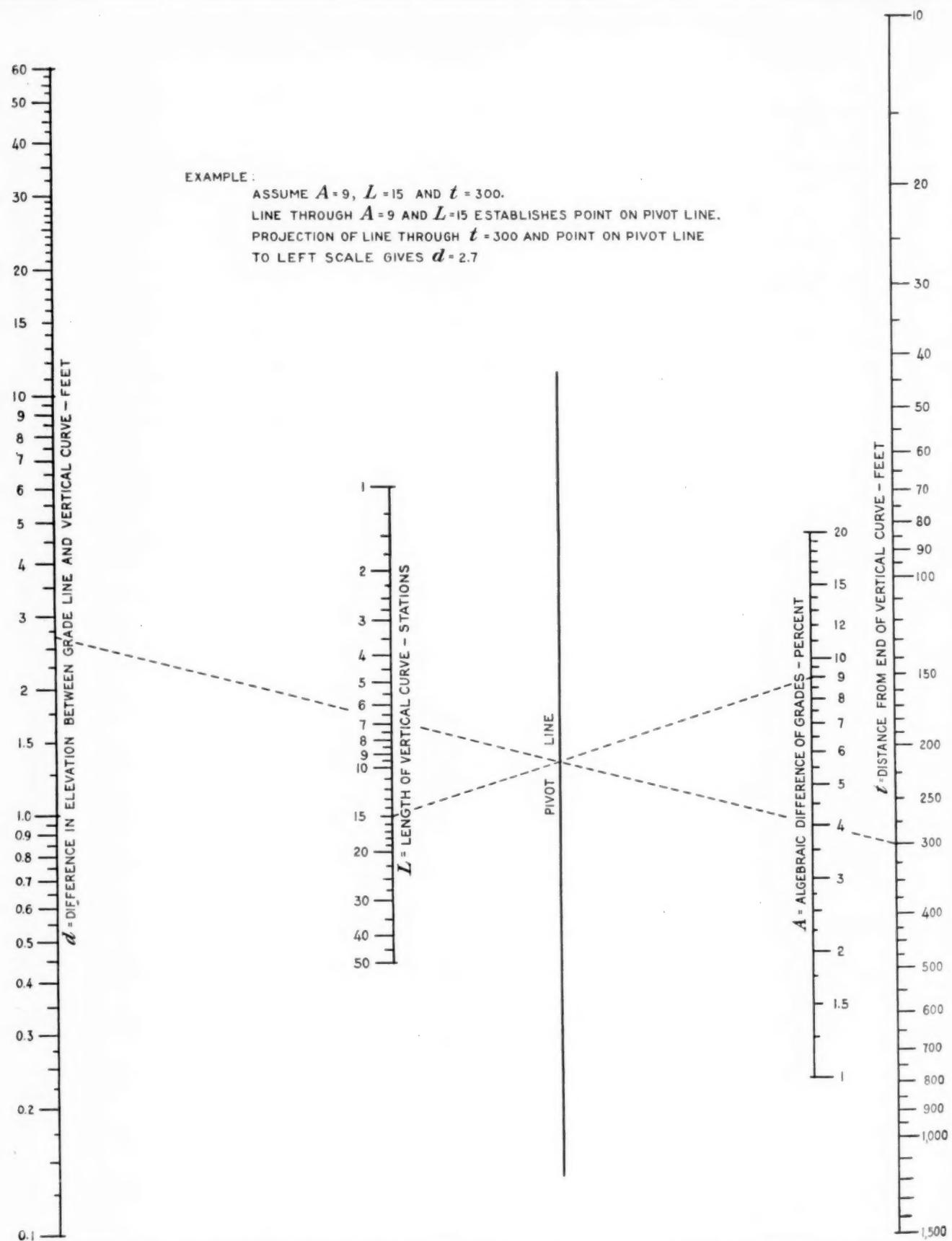


FIGURE 2.—NOMOGRAPH FOR CALCULATION OF ELEVATION OF A POINT ON A PARABOLIC VERTICAL CURVE.

TABLE 3.—Values of  $\frac{A}{L}$ , for values of  $A$  from 1 to 16 and values of  $L$  from 1 to 30

$L$ (stations)	$A=1$ percent	$A=2$ percent	$A=3$ percent	$A=4$ percent	$A=5$ percent	$A=6$ percent	$A=7$ percent	$A=8$ percent	$A=9$ percent	$A=10$ percent	$A=11$ percent	$A=12$ percent	$A=13$ percent	$A=14$ percent	$A=15$ percent	$A=16$ percent
1	1.0000	2.0000	3.0000	4.0000	5.0000	6.0000	7.0000	8.0000	9.0000	10.0000	11.0000	12.0000	13.0000	14.0000	15.0000	16.0000
2	.5000	1.0000	1.5000	2.0000	2.5000	3.0000	3.5000	4.0000	4.5000	5.0000	5.5000	6.0000	6.5000	7.0000	7.5000	8.0000
3	.3333	.6667	1.0000	1.3333	1.6667	2.0000	2.3333	2.6667	3.0000	3.3333	3.6667	4.0000	4.3333	4.6667	5.0000	5.3333
4	.2500	.5000	.7500	1.0000	1.2500	1.5000	1.7500	2.0000	2.2500	2.5000	2.7500	3.0000	3.2500	3.5000	3.7500	4.0000
5	.2000	.4000	.6000	.8000	1.0000	1.2000	1.4000	1.6000	1.8000	2.0000	2.2000	2.4000	2.6000	2.8000	3.0000	3.2000
6	.1667	.3333	.5000	.6667	.8333	1.0000	1.1667	1.3333	1.5000	1.6667	1.8333	2.0000	2.1667	2.3333	2.5000	2.6667
7	.1428	.2857	.4286	.5714	.7143	.8571	1.0000	1.1428	1.2857	1.4286	1.5714	1.7143	1.8571	2.0000	2.1428	2.2857
8	.1250	.2500	.3750	.5000	.6250	.7500	.8750	1.0000	1.1250	1.2500	1.3750	1.5000	1.6250	1.7500	1.8750	2.0000
9	.1111	.2222	.3333	.4444	.5556	.6667	.7778	.8889	1.0000	1.1111	1.2222	1.3333	1.4444	1.5556	1.6667	1.7778
10	.1000	.2000	.3000	.4000	.5000	.6000	.7000	.8000	.9000	1.0000	1.1000	1.2000	1.3000	1.4000	1.5000	1.6000
11	.0909	.1818	.2727	.3636	.4545	.5454	.6364	.7273	.8182	.9091	1.0000	1.0909	1.1818	1.2727	1.3636	1.4545
12	.0833	.1667	.2500	.3333	.4167	.5000	.5833	.6667	.7500	.8333	.9167	1.0000	1.0833	1.1667	1.2500	1.3333
13	.0769	.1538	.2308	.3077	.3846	.4615	.5385	.6154	.6923	.7692	.8462	.9231	1.0000	1.0769	1.1538	1.2308
14	.0714	.1428	.2143	.2857	.3571	.4286	.5000	.5714	.6428	.7143	.7857	.8571	.9286	1.0000	1.0714	1.1428
15	.0667	.1333	.2000	.2667	.3333	.4000	.4667	.5333	.6000	.6667	.7333	.8000	.8667	.9333	1.0000	1.0667
16	.0625	.1250	.1875	.2500	.3125	.3750	.4375	.5000	.5625	.6250	.6875	.7500	.8125	.8750	.9375	1.0000
17	.0588	.1176	.1765	.2353	.2941	.3529	.4118	.4706	.5294	.5882	.6470	.7059	.7647	.8235	.8823	.9412
18	.0556	.1111	.1667	.2222	.2778	.3333	.3889	.4444	.5000	.5556	.6111	.6667	.7222	.7778	.8333	.8889
19	.0526	.1053	.1579	.2105	.2632	.3158	.3684	.4210	.4737	.5263	.5789	.6316	.6842	.7368	.7894	.8421
20	.0500	.1000	.1500	.2000	.2500	.3000	.3500	.4000	.4500	.5000	.5500	.6000	.6500	.7000	.7500	.8000
21	.0476	.0952	.1428	.1905	.2381	.2857	.3333	.3809	.4286	.4762	.5238	.5714	.6190	.6667	.7143	.7619
22	.0454	.0909	.1364	.1818	.2273	.2727	.3182	.3636	.4091	.4545	.5000	.5454	.5909	.6364	.6818	.7273
23	.0434	.0870	.1304	.1739	.2174	.2609	.3043	.3478	.3913	.4348	.4783	.5217	.5652	.6087	.6522	.6956
24	.0416	.0833	.1250	.1667	.2083	.2500	.2917	.3333	.3750	.4167	.4583	.5000	.5417	.5833	.6250	.6667
25	.0400	.0800	.1200	.1600	.2000	.2400	.2800	.3200	.3600	.4000	.4400	.4800	.5200	.5600	.6000	.6400
26	.0385	.0769	.1154	.1538	.1923	.2308	.2692	.3077	.3462	.3846	.4231	.4615	.5000	.5385	.5769	.6154
27	.0370	.0740	.1111	.1481	.1852	.2222	.2593	.2963	.3333	.3704	.4074	.4444	.4815	.5185	.5555	.5926
28	.0357	.0714	.1071	.1428	.1786	.2143	.2500	.2857	.3214	.3571	.3928	.4286	.4643	.5000	.5357	.5714
29	.0345	.0690	.1034	.1379	.1724	.2069	.2414	.2759	.3103	.3448	.3793	.4138	.4483	.4828	.5172	.5517
30	.0333	.0667	.1000	.1333	.1667	.2000	.2333	.2667	.3000	.3333	.3667	.4000	.4333	.4667	.5000	.5333

TABLE 4.—Values of  $K$  for values of  $t$  from 1 to 1,500

$t$	$K$	$t$	$K$	$t$	$K$	$t$	$K$	$t$	$K$	$t$	$K$	$t$	$K$	$t$	$K$
1	0.000	51	0.130	101	0.510	151	1.140	201	2.020	251	3.150	301	4.530	351	6.160
2	.000	.52	.135	102	.520	152	1.155	202	2.040	252	3.175	302	4.560	352	6.195
3	.000	.53	.140	103	.530	153	1.170	203	2.060	253	3.200	303	4.590	353	6.230
4	.001	.54	.146	104	.541	154	1.186	204	2.081	254	3.226	304	4.621	354	6.266
5	.001	.55	.151	105	.551	155	1.201	205	2.101	255	3.251	305	4.651	355	6.301
6	.002	.56	.157	106	.562	156	1.217	206	2.122	256	3.277	306	4.682	356	6.337
7	.002	.57	.162	107	.572	157	1.232	207	2.142	257	3.302	307	4.712	357	6.372
8	.003	.58	.168	108	.583	158	1.248	208	2.163	258	3.328	308	4.743	358	6.408
9	.004	.59	.174	109	.594	159	1.264	209	2.184	259	3.354	309	4.774	359	6.444
10	.005	.60	.180	110	.605	160	1.280	210	2.205	260	3.380	310	4.805	360	6.480
11	.006	.61	.186	111	.616	161	1.296	211	2.226	261	3.406	311	4.836	361	6.516
12	.007	.62	.192	112	.627	162	1.312	212	2.247	262	3.432	312	4.867	362	6.552
13	.008	.63	.198	113	.638	163	1.328	213	2.268	263	3.458	313	4.898	363	6.588
14	.010	.64	.205	114	.650	164	1.345	214	2.290	264	3.485	314	4.930	364	6.625
15	.011	.65	.211	115	.661	165	1.361	215	2.311	265	3.511	315	4.961	365	6.661
16	.013	.66	.218	116	.673	166	1.378	216	2.333	266	3.538	316	4.993	366	6.698
17	.014	.67	.224	117	.684	167	1.394	217	2.354	267	3.564	317	5.024	367	6.734
18	.016	.68	.231	118	.696	168	1.411	218	2.376	268	3.591	318	5.056	368	6.771
19	.018	.69	.238	119	.708	169	1.428	219	2.398	269	3.618	319	5.088	369	6.808
20	.020	.70	.245	120	.720	170	1.445	220	2.420	270	3.645	320	5.120	370	6.845
21	.022	.71	.252	121	.732	171	1.462	221	2.442	271	3.672	321	5.152	371	6.882
22	.024	.72	.259	122	.744	172	1.479	222	2.464	272	3.699	322	5.184	372	6.919
23	.026	.73	.266	123	.756	173	1.496	223	2.486	273	3.726	323	5.216	373	6.956
24	.029	.74	.274	124	.769	174	1.514	224	2.509	274	3.754	324	5.249	374	6.994
25	.031	.75	.281	125	.781	175	1.531	225	2.531	275	3.781	325	5.281	375	7.031
26	.034	.76	.289	126	.794	176	1.549	226	2.554	276	3.809	326	5.314	376	7.069
27	.036	.77	.296	127	.806	177	1.566	227	2.576	277	3.836	327	5.346	377	7.106
28	.039	.78	.304	128	.819	178	1.584	228	2.599	278	3.864	328	5.379	378	7.144
29	.042	.79	.312	129	.832	179	1.602	229	2.622	279	3.892	329	5.412	379	7.182
30	.045	.80	.320	130	.845	180	1.620	230	2.645	280	3.920	330	5.445	380	7.220
31	.048	.81	.328	131	.858	181	1.638	231	2.668	281	3.948	331	5.478	381	7.258
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TABLE 4.—Values of  $K$  for values of  $t$  from 1 to 1,500—Con.

$t$	$K$	$t$	$K$	$t$	$K$	$t$	$K$
401	8.040	501	12.550	601	18.060	701	24.570
402	8.080	502	12.600	602	18.120	702	24.640
403	8.120	503	12.650	603	18.180	703	24.710
404	8.161	504	12.701	604	18.241	704	24.781
405	8.201	505	12.751	605	18.301	705	24.851
406	8.242	506	12.802	606	18.362	706	24.922
407	8.282	507	12.852	607	18.422	707	24.992
408	8.323	508	12.903	608	18.483	708	25.063
409	8.364	509	12.954	609	18.544	709	25.134
410	8.405	510	13.005	610	18.605	710	25.205
411	8.446	511	13.056	611	18.666	711	25.276
412	8.487	512	13.107	612	18.727	712	25.347
413	8.528	513	13.158	613	18.788	713	25.418
414	8.570	514	13.210	614	18.850	714	25.490
415	8.611	515	13.261	615	18.911	715	25.561
416	8.653	516	13.313	616	18.973	716	25.633
417	8.694	517	13.364	617	19.034	717	25.704
418	8.736	518	13.416	618	19.096	718	25.776
419	8.778	519	13.468	619	19.158	719	25.848
420	8.820	520	13.520	620	19.220	720	25.920
421	8.862	521	13.572	621	19.282	721	25.992
422	8.904	522	13.624	622	19.344	722	26.064
423	8.946	523	13.676	623	19.406	723	26.136
424	8.989	524	13.729	624	19.469	724	26.209
425	9.031	525	13.781	625	19.531	725	26.281
426	9.074	526	13.834	626	19.594	726	26.354
427	9.116	527	13.886	627	19.656	727	26.426
428	9.159	528	13.939	628	19.719	728	26.499
429	9.202	529	13.992	629	19.782	729	26.572
430	9.245	530	14.045	630	19.845	730	26.645
431	9.288	531	14.098	631	19.908	731	26.718
432	9.331	532	14.151	632	19.971	732	26.791
433	9.374	533	14.204	633	20.034	733	26.864
434	9.418	534	14.258	634	20.098	734	26.938
435	9.461	535	14.311	635	20.161	735	27.011
436	9.505	536	14.365	636	20.225	736	27.085
437	9.548	537	14.418	637	20.288	737	27.158
438	9.592	538	14.472	638	20.352	738	27.232
439	9.636	539	14.526	639	20.416	739	27.306
440	9.680	540	14.580	640	20.480	740	27.380
441	9.724	541	14.634	641	20.544	741	27.454
442	9.768	542	14.688	642	20.608	742	27.528
443	9.812	543	14.742	643	20.672	743	27.602
444	9.857	544	14.797	644	20.737	744	27.677
445	9.901	545	14.851	645	20.801	745	27.751
446	9.946	546	14.906	646	20.866	746	27.826
447	9.990	547	14.960	647	20.930	747	27.900
448	10.035	548	15.015	648	20.995	748	27.975
449	10.080	549	15.070	649	21.060	749	28.050
450	10.125	550	15.125	650	21.125	750	28.125
451	10.170	551	15.180	651	21.190	751	28.200
452	10.215	552	15.235	652	21.255	752	28.275
453	10.260	553	15.290	653	21.320	753	28.350
454	10.306	554	15.346	654	21.386	754	28.426
455	10.351	555	15.401	655	21.451	755	28.501
456	10.397	556	15.457	656	21.517	756	28.577
457	10.442	557	15.512	657	21.582	757	28.652
458	10.488	558	15.568	658	21.648	758	28.728
459	10.534	559	15.624	659	21.714	759	28.804
460	10.580	560	15.680	660	21.780	760	28.880
461	10.626	561	15.736	661	21.846	761	28.956
462	10.672	562	15.792	662	21.912	762	29.032
463	10.718	563	15.848	663	21.978	763	29.108
464	10.765	564	15.905	664	22.045	764	29.185
465	10.811	565	15.961	665	22.111	765	29.261
466	10.858	566	16.018	666	22.178	766	29.338
467	10.904	567	16.074	667	22.244	767	29.414
468	10.951	568	16.131	668	22.311	768	29.491
469	10.998	569	16.188	669	22.378	769	29.568
470	11.045	570	16.245	670	22.445	770	29.645
471	11.092	571	16.302	671	22.512	771	29.722
472	11.139	572	16.359	672	22.579	772	29.799
473	11.186	573	16.416	673	22.646	773	29.876
474	11.234	574	16.474	674	22.714	774	29.954
475	11.281	575	16.531	675	22.781	775	30.031
476	11.329	576	16.589	676	22.849	776	30.109
477	11.376	577	16.646	677	22.916	777	30.186
478	11.424	578	16.704	678	22.984	778	30.264
479	11.472	579	16.762	679	23.052	779	30.342
480	11.520	580	16.820	680	23.120	780	30.420
481	11.568	581	16.878	681	23.188	781	30.498
482	11.616	582	16.936	682	23.256	782	30.576
483	11.664	583	16.994	683	23.324	783	30.654
484	11.713	584	17.053	684	23.393	784	30.733
485	11.761	585	17.111	685	23.461	785	30.811
486	11.810	586	17.170	686	23.530	786	30.890
487	11.858	587	17.228	687	23.598	787	30.968
488	11.907	588	17.287	688	23.667	788	31.047
489	11.956	589	17.346	689	23.736	789	31.126
490	12.005	590	17.405	690	23.805	790	31.205
491	12.054	591	17.464	691	23.874	791	31.284
492	12.103	592	17.523	692	23.943	792	31.363
493	12.152	593	17.582	693	24.012	793	31.442
494	12.202	594	17.642	694	24.082	794	31.522
495	12.251	595	17.701	695	24.151	795	31.601
496	12.301	596	17.761	696	24.221	796	31.681
497	12.350	597	17.820	697	24.290	797	31.760
498	12.400	598	17.880	698	24.360	798	31.840
499	12.450	599	17.940	699	24.430	799	31.920
500	12.500	600	18.000	700	24.500	800	32.000

TABLE 4.—Values of  $K$  for values of  $t$  from 1 to 1,500—Con.

$t$	$K$	$t$	$K$	$t$	$K$	$t$	$K$	$t$	$K$
801	32.080	901	40.590	1,001	50.100	1,101	60.610	1,201	60.610
802	32.160	902	40.680	1,002	50.200	1,102	60.720	1,202	60.720
803	32.240	903	40.770	1,003	50.300	1,103	60.830	1,203	60.830
804	32.321	904	40.861	1,004	50.401	1,104	60.940	1,204	60.940
805	32.401	905	40.951	1,005	50.501	1,105	61.051	1,205	61.051
806	32.482	906	41.042	1,006	50.602	1,106	61.162	1,206	61.162
807	32.562	907	41.132	1,007	50.702	1,107	61.272	1,207	61.272
808	32.643	908	41.223	1,008	50.803	1,108	61.382	1,208	61.382
809	32.724	909	41.312	1,009	50.904	1,109	61.494	1,209	61.494
810	32.805	910	41.405	1,010	51.005	1,110	61.605	1,210	61.605
811	32.886	911	41.496	1,011	51.106	1,111	61.716	1,211	61.716
812	32.967	912	41.587	1,012	51.207	1,112	61.827	1,212	61.827
813	33.048	913	41.678	1,013	51.308	1,113	61.938	1,213	61.938
814	33.129	914	41.769	1,014	51.409	1,114	62.050	1,214	62.050
815	33.210	915	41.861	1,015	51.511	1,115	62.161	1,215	62.161
816	33.291	916	41.953	1,016	51.613	1,116	62.273	1,216	62.273
817	33.372	917	42.044	1,017	51.714	1,117	62.384	1,217	62.384
818	33.453	918	42.135						

TABLE 4.—Values of  $K$  for values of  $t$  from 1 to 1,500—Con.

$t$	$K$	$t$	$K$	$t$	$K$	$t$	$K$
1,201	72.120	1,246	77.626	1,291	83.334	1,336	89.245
1,202	72.240	1,247	77.750	1,292	83.463	1,337	89.378
1,203	72.360	1,248	77.875	1,293	83.592	1,338	89.512
1,204	72.481	1,249	78.000	1,294	83.722	1,339	89.646
1,205	72.601	1,250	78.125	1,295	83.851	1,340	89.780
1,206	72.722			1,296	83.981		
1,207	72.842	1,251	78.250	1,297	84.110	1,341	89.914
1,208	72.963	1,252	78.375	1,298	84.240	1,342	90.048
1,209	73.084	1,253	78.500	1,299	84.370	1,343	90.182
1,210	73.205	1,254	78.625	1,300	84.500	1,344	90.317
			1,255	78.751		1,345	90.451
1,211	73.326	1,256	78.877	1,301	84.630	1,346	90.586
1,212	73.447	1,257	79.002	1,302	84.760	1,347	90.720
1,213	73.568	1,258	79.128	1,303	84.890	1,348	90.855
1,214	73.689	1,259	79.254	1,304	85.021	1,349	90.990
1,215	73.811	1,260	79.380	1,305	85.151	1,350	91.125
1,216	73.933			1,306	85.282		
1,217	74.054	1,261	79.506	1,307	85.412	1,351	91.260
1,218	74.176	1,262	79.632	1,308	85.543	1,352	91.395
1,219	74.298	1,263	79.758	1,309	85.674	1,353	91.530
1,220	74.420	1,264	79.885	1,310	85.805	1,354	91.666
			1,265	80.011		1,355	91.801
1,221	74.542	1,266	80.138	1,311	85.936	1,356	91.937
1,222	74.664	1,267	80.264	1,312	86.067	1,357	92.072
1,223	74.786	1,268	80.391	1,313	86.198	1,358	92.208
1,224	74.909	1,269	80.518	1,314	86.330	1,359	92.344
1,225	75.031	1,270	80.645	1,315	86.461	1,360	92.480
1,226	75.154			1,316	86.593		
1,227	75.276	1,271	80.772	1,317	86.724	1,361	92.616
1,228	75.399	1,272	80.899	1,318	86.856	1,362	92.752
1,229	75.522	1,273	81.026	1,319	86.988	1,363	92.888
1,230	75.645	1,274	81.154	1,320	87.120	1,364	93.025
			1,275	81.281		1,365	93.161
1,231	75.768	1,276	81.409	1,321	87.252	1,366	93.298
1,232	75.891	1,277	81.536	1,322	87.384	1,367	93.434
1,233	76.014	1,278	81.664	1,323	87.516	1,368	93.571
1,234	76.138	1,279	81.792	1,324	87.649	1,369	93.708
1,235	76.261	1,280	81.920	1,325	87.781	1,370	93.845
1,236	76.385			1,326	87.914		
1,237	76.508	1,281	82.048	1,327	88.046	1,371	93.982
1,238	76.632	1,282	82.176	1,328	88.179	1,372	94.119
1,239	76.756	1,283	82.304	1,329	88.312	1,373	94.256
1,240	76.880	1,284	82.433	1,330	88.445	1,374	94.393
			1,285	82.561		1,375	94.531
1,241	77.004	1,286	82.690	1,331	88.578	1,376	94.669
1,242	77.128	1,287	82.818	1,332	88.711	1,377	94.806
1,243	77.252	1,288	82.947	1,333	88.844	1,378	94.944
1,244	77.377	1,289	83.076	1,334	88.978	1,379	95.082
1,245	77.501	1,290	83.205	1,335	89.111	1,380	95.220

TABLE 4.—Values of  $K$  for values of  $t$  from 1 to 1,500—Con.

$t$	$K$	$t$	$K$	$t$	$K$	$t$	$K$
1,381	95.358	1,411	99.546	1,441	103.824	1,471	108.192
1,382	95.496	1,412	99.687	1,442	103.968	1,472	108.339
1,383	95.634	1,413	99.828	1,443	104.112	1,473	108.486
1,384	95.773	1,414	99.970	1,444	104.257	1,474	108.634
1,385	95.911	1,415	100.111	1,445	104.401	1,475	108.781
1,386	96.050	1,416	100.253	1,446	104.546	1,476	108.929
1,387	96.188	1,417	100.394	1,447	104.690	1,477	109.076
1,388	96.327	1,418	100.536	1,448	104.835	1,478	109.224
1,389	96.466	1,419	100.678	1,449	104.980	1,479	109.372
1,390	96.605	1,420	100.820	1,450	105.125	1,480	109.520

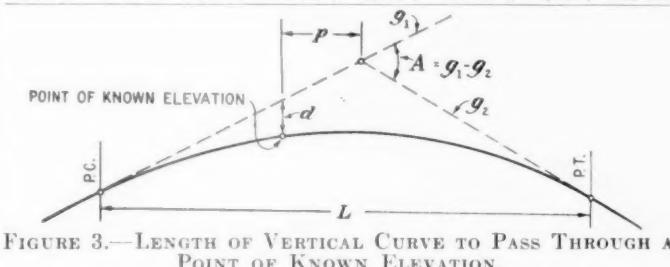
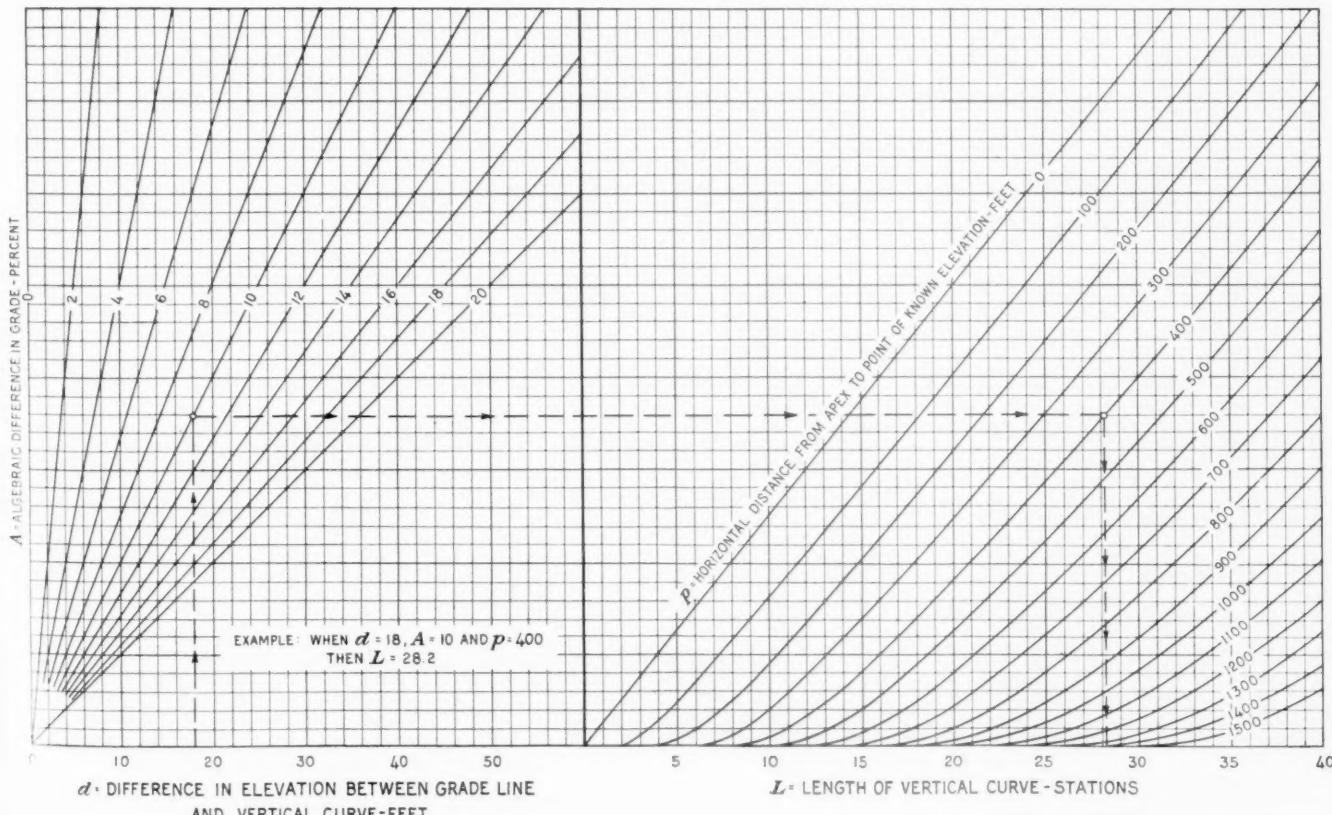


FIGURE 3.—LENGTH OF VERTICAL CURVE TO PASS THROUGH A POINT OF KNOWN ELEVATION.



PROJECT 121-C							
Station	Grade, percent, $\delta$	Elevation on grade line	$t$ (from table 3)	$k$ (from table 2)	$d$ (from A-L-E)	Elevation	
						Ground	Grade
86+00	10.50	120.25	—	—	—	120.4	120.25
+50	PC	120.50	0	0	0	121.0	120.50
87+00	120.75	50	0.125	0.05	121.4	120.75	0.7
+50	121.00	100	0.500	0.21	121.6	120.79	0.8
87+50	121.25	150	1.125	0.47	121.8	120.78	1.0
+50	121.50	200	2.000	0.83	121.5	120.67	0.8
88+00	121.75	250	3.125	1.30	120.9	120.45	0.4
+50	PT	122.00	300	4.500	1.88	120.5	120.12
89+00	121.00	250	3.125	1.30	119.6	119.70	—
+50	120.00	200	2.000	0.83	118.4	119.17	—
85+00	119.00	150	1.125	0.47	117.9	118.53	—
+50	118.00	100	0.500	0.21	117.4	117.79	—
86+00	117.00	50	0.125	0.05	116.7	116.95	—
+50	PT	116.00	0	0	116.2	116.00	0.2
87+00	115.00	—	—	—	116.7	115.00	1.7

FIGURE 5.—SAMPLE SHEET FOR CALCULATION OF A VERTICAL CURVE.

Length of vertical curve to pass through a point of known elevation. Values of the following are known (see fig. 3):

$A$ =algebraic difference of grades, percent;  
 $p$ =horizontal distance from apex to point of known elevation, feet; and  
 $d$ =difference in elevation between grade line and vertical curve at point of known elevation, feet.

It is desired to determine  $L$ , the length of vertical curve (in stations) to pass through point of known elevation. The following basic formula is used:

$$d = \frac{A (50L - p)^2}{20,000 L}$$

A direct graphical solution for value of  $L$  is given in figure 4. The steps in using this figure are:

1. Enter at value of  $d$  (lower left scale) and project upward to value of  $A$  (left sloping lines).

2. Project horizontally across to value of  $p$  (right sloping lines).

3. Project down to read value of  $L$  (lower right scale).

Figure 4 may also be used to solve for value of  $d$  when values of  $A$ ,  $L$ , and  $p$  are known.

Figure 5 is a sample sheet for calculation of a vertical curve using the values of tables 3 and 4.

## COTTON FABRIC FOR EROSION CONTROL

### REPORT OF EXPERIMENTS ON ROADSIDE SLOPES AND DITCHES IN MISSISSIPPI

#### BY DISTRICT 8, PUBLIC ROADS ADMINISTRATION

Reported by W. B. KING, Associate Bridge Construction Engineer

COTTON FABRIC was used experimentally for erosion control on roadside slopes and ditches on eight test sections in Mississippi during the summer and fall of 1940. The fabric was furnished by the Division of Marketing and Marketing Agreements of the United States Department of Agriculture, and was installed by the Mississippi State Highway Department.

The fabric used was of the following types: S1-54, S1-40, S2-54, and S2-40, the latter figure in each designation being the width of the fabric in inches. Approximately equal amounts of each type of fabric were used, 15,948 square yards being used in the eight test sections. Both types of cotton fabric have relatively large mesh openings, type S1 having 6 openings per inch and type S2 having 4. Table 1 shows the amount of each type of fabric and use on each test section.

The fabric was delivered in rolls. On some areas of the test sections the fabric was run horizontally; on others it was run from top to toe of the slope, or "vertically."

Descriptions of each test section and the results obtained follow:

**Section A.**—The soil was sand-clay, clay, and marl. The areas were first treated by spreading a mixture of bermuda grass roots and topsoil to a depth of approximately 2 inches. The grass mulch was spread uniformly

with shovels and rakes. The fabric was then applied and pegged down with galvanized wire staples. After passing through one growing season there appeared to be little difference in the amount of growth on the covered and uncovered areas. The fabric-covered slope was greener and the ground somewhat more moist than was the uncovered slope. The fabric in the ditch bottom appeared definitely to have retarded erosion.

Figure 1 shows the appearance of portions of Section A soon after the fabric was placed and the appearance of the same portions after one growing season.

TABLE 1.—AMOUNT OF EACH TYPE OF COTTON FABRIC AND USE ON EACH TEST SECTION

Section No.	Project No.	Amount of cotton fabric, type—		Used on—
		S1	S2	
A	PWS 72-C	Sq. yd.	Sq. yd.	3:1 to 6:1 slopes; ditch bottom.
B	PWS 123	1,305	750	2:1 backslopes.
C	PWS 82-C	1,500	2,250	2½:1 and 4:1 backslopes; ditch bottom.
		749	999	
D	NRH 74-G	1,361	109	2½:1 backslopes; ditch bottom.
E	PWA 254-A	1,200	2,125	3:1 backslope; ditch bottom.
F	FAP 173-A	1,100	1,105	1½:1 and 2:1 backslopes.
G	NRS 236	555	750	2:1 backslopes.
H	FAP 45	500	500	2:1 backslopes.
Total.		8,270	7,678	

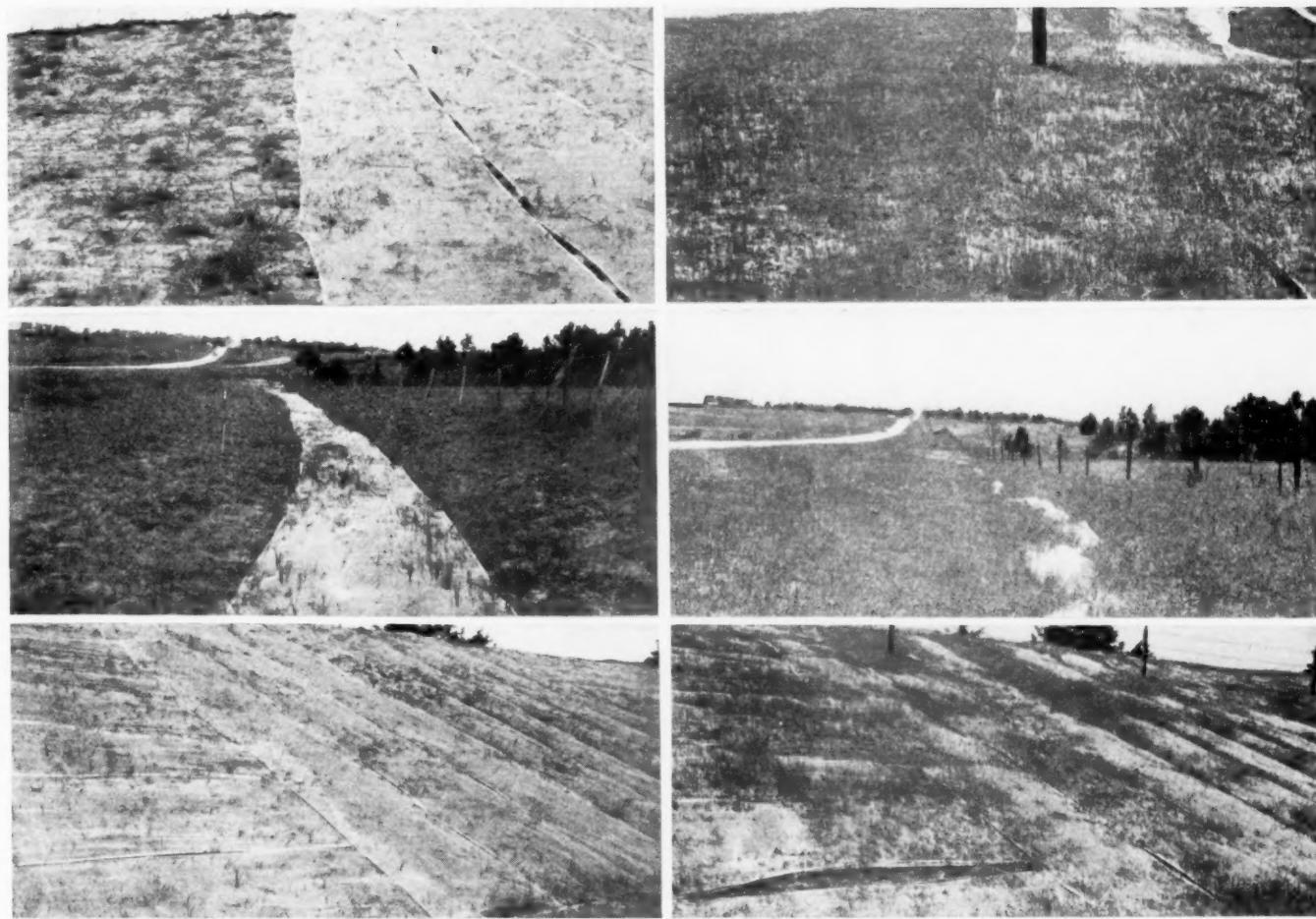


FIGURE 1.—LEFT, APPEARANCE OF PORTIONS OF SECTION A SOON AFTER FABRIC WAS PLACED; RIGHT, APPEARANCE OF THE SAME RESPECTIVE PORTIONS OF SECTION A AFTER ONE GROWING SEASON.

**Section B.**—The soil was sand, sand-clay, and clay. The backslopes were treated the same as the above section; and in addition commercial 4-8-4 fertilizer was mixed into the topsoil at the rate of 750 pounds per acre, and bermuda and carpet grass seed were planted. Part of the treated area was covered with cotton fabric pegged down with wooden stakes, and part was covered with straw mulch. After one growing season the fabric appeared to have helped the growth by shading and retarding erosion. However, the straw mulch appeared to have given better results more cheaply.

**Section C.**—The soil was topsoil, clay, sand-clay, and rock. The 2½:1 slopes and the ditch bottom were treated with topsoil and grass roots the same as on Section A. The 4:1 slope, consisting of clay and sand-clay soil, received no treatment except the cotton fabric covering. The fabric was pegged down with galvanized wire staples. After one growing season, the fabric in the ditch bottom appeared definitely to have retarded erosion and aided the growth. The fabric on the 2½:1 slopes helped by shading the growth. No difference was noted in the amount of erosion on the covered and uncovered portions. On the 4:1 slope, where no grass or topsoil was provided, the fabric appeared definitely to have retarded erosion and promoted native growth.

**Section D.**—The soil was a mixture of sand and clay. The areas were treated with topsoil and grass roots the same as on Section A, and the fabric similarly pegged

with galvanized wire staples. At the time of inspection after one growing season, a large portion of the fabric on the slopes had disappeared. No appreciable difference could be noted in the portion of the slopes that was originally covered with fabric and the portion that was left uncovered. The fabric in the ditch bottom appeared to have helped retard erosion.

Figure 2 shows the appearance of portions of Sections B, C, and D after the fabric was placed, and the appearance of the same sections after one growing season.

**Section E.**—The soil was a sandy clay. A short time prior to placing the fabric the slopes were plowed, fertilized, seeded with grass, and sprigged. The grass had attained a fair growth prior to applying the fabric. A short time after the fabric was placed, two-thirds of it was destroyed by fire. No appreciable difference was noted between areas where the bank remained covered and where it was uncovered. Some erosion was noted between the grass rows beneath the fabric that remained in place. The fabric helped to retard erosion in the ditch bottom.

**Section F.**—The soil was limestone, hard-pan, sand-clay, and clay. The fabric was placed on backslopes that had been strip-sodded about 2 years previously. Some fabric was used on backslopes without sod. The fabric was pegged down with wooden stakes. Nine months after installation an inspection revealed that on the upper part of the north bank growth had benefited



FIGURE 2.—LEFT, APPEARANCE OF PORTIONS OF SECTIONS B (TOP), C (MIDDLE), AND D (BOTTOM) DURING OR SOON AFTER PLACING OF FABRIC; RIGHT, APPEARANCE OF THE SAME RESPECTIVE PORTIONS AFTER ONE GROWING SEASON.

by the shade afforded by the fabric. No benefit was noted on the limestone and clay soil at the bottom of the north bank. On the south bank the growth under the fabric appeared to have benefited by the shade, it being greener than the uncovered portions. No other difference between the covered and uncovered portions was noted.

*Section G.*—The slopes had been covered with 4 inches of good topsoil containing bermuda and other native grasses. The grass had attained a fair growth before the fabric was placed. However, the results could not be compared with those for other sections because on two of the banks the fabric had been covered with straw mulch and on the third bank the fabric had been removed and the bank covered with straw mulch.

*Section H.*—The soil in the slopes was sand-clay. All of the fabric was placed on sodded slopes where the grass was already well established. No results could be obtained because all of the fabric was torn loose by cattle and hogs a short time after it was placed.

The results obtained on these eight experimental sections indicate that:

1. There is no apparent advantage in using cotton fabric on 3:1 or flatter slopes that have been treated

with a mixture of grass roots and topsoil.

2. Use of cotton fabric has some advantage in shading growth and retarding erosion on 2:1 and 1½:1 slopes that have been treated with a mixture of grass roots and topsoil, or sprigged, or strip sodded. However, straw mulch serves better and is more economical wherever available.

3. Cotton fabric aids natural growth and retards erosion on 4:1 slopes of sandy clay and clay soils that have received no other treatment. However, straw mulch serves better wherever available.

4. There is definite advantage in using fabric in roadside ditch bottoms in promoting growth and retarding erosion. The fabric should be pegged at short intervals across its width to hold it close to the ground.

5. The S2-54 fabric is more effective than the other types. The open mesh of the S2 fabric allows vegetation to grow through it, and the wider material is more economical to install per square yard.

6. For greatest effectiveness, the fabric should be placed up and down on slopes and parallel to the flow line in ditches.

7. Galvanized wire staples are more effective than wooden stakes in pegging the fabric.

(Continued from page 88)

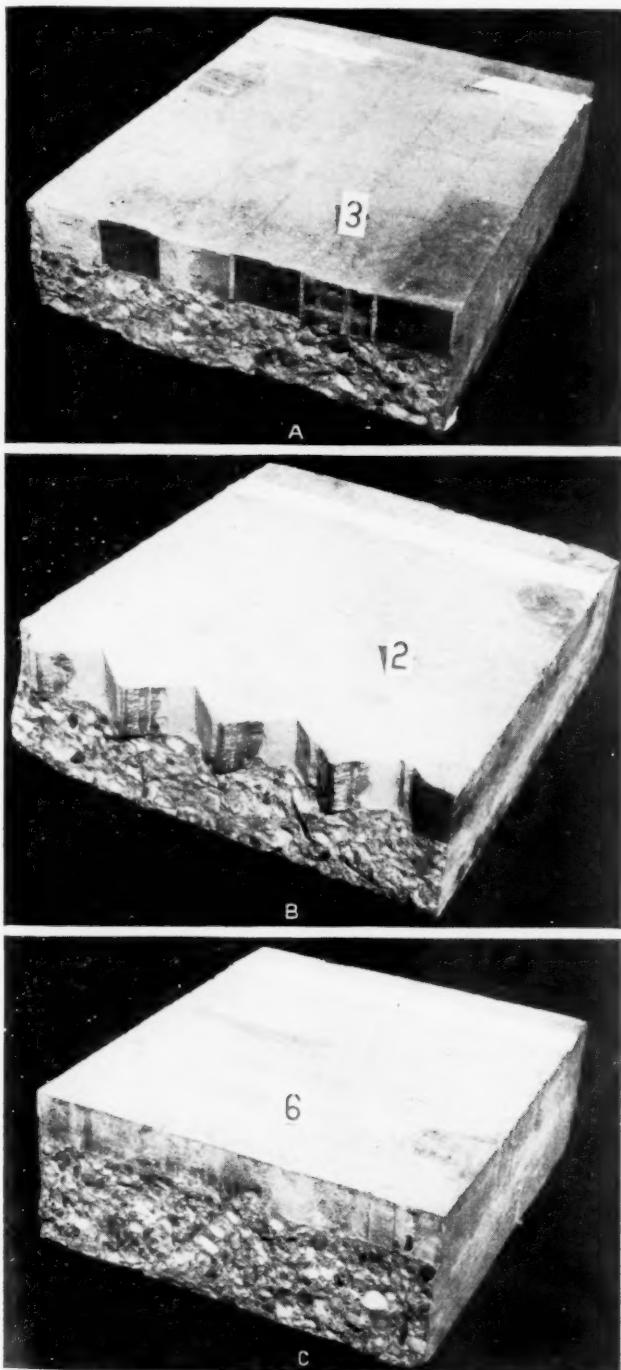


FIGURE 4.—MONOLITHIC BRICK SLABS TESTED WITH BRICK SURFACE IN TENSION. BRICK COURSES LAID (A) PERPENDICULAR, (B) AT AN ANGLE OF 45°, AND (C) PARALLEL TO THE AXIS OF THE LOADING KNIFE BLADE. NOTE FAILURE BETWEEN BRICK AND MORTAR IN (C).

perpendicular to the axis of the loading knife edge exhibited strengths equal to about 90 percent of the strength of plain concrete slabs of equal thickness, while the strength of the slabs in which the brick were set at a 45° angle averaged 67 percent of the strength of plain concrete slabs. In these tests the concrete used in the monolithic brick slabs was of the same quality (6 sacks of cement per cubic yard) as that used in the plain concrete slabs.

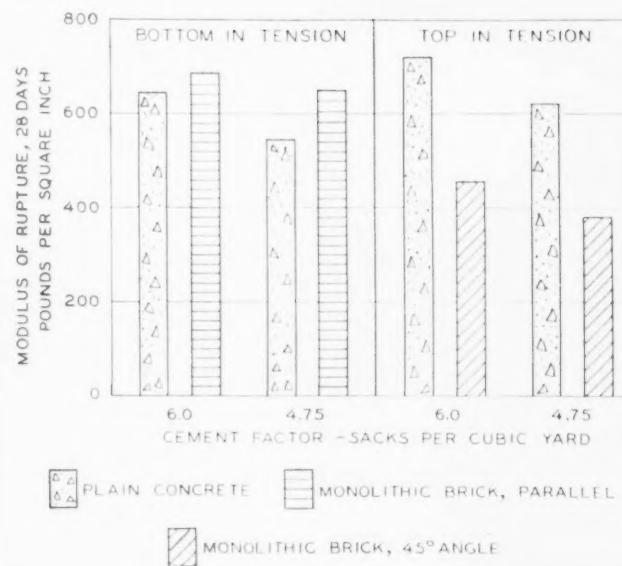


FIGURE 5.—RESULTS OF FLEXURE TESTS ON MONOLITHIC BRICK PAVEMENT SLABS, SERIES B, AT AGE OF 28 DAYS.

Strength results for series B are shown in table 7 and in figure 5. In this series all specimens were 8 inches thick. However, two different classes of concrete were used; one the pavement mix (6 sacks of cement per cubic yard), the other the base mix (4½ sacks of cement per cubic yard). Strength results for slabs tested with the bottom, as cast, in tension are shown in the left panel of figure 5. It will be observed that, for both the pavement and base concretes, the monolithic brick slabs gave higher strengths than did the plain specimens, the average differential in strength being 13 percent. Referring to the results for series A as given in table 6, it will be observed that the differential in favor of the 8-inch monolithic brick slabs, tested with the bottom in tension was also 13 percent.

TABLE 7.—Summary of results of flexure tests on monolithic brick pavement slabs, series B<sup>1</sup>

Structure	Cement factor	Surface in tension	Direction of knife edge of applied load <sup>2</sup>	Flexural strength <sup>3</sup>	Strength ratio <sup>4</sup>
Concrete	Sacks per cu. yd.			Lb. per sq. in.	Percent
Concrete	6.0	Bottom		646	100
Monolithic brick	6.0	do	Parallel	686	106
Concrete	4.75	do	Parallel	545	100
Monolithic brick	4.75	do	Parallel	630	119
Concrete	6.0	Top		721	100
Monolithic brick	6.0	do	45°	454	63
Concrete	4.75	do	45°	622	100
Monolithic brick	4.75	do	45°	379	61

<sup>1</sup> Specimens 26 inches wide by 60 inches long by 8 inches thick (approximately); tested with center loading on a 54-inch span after 28 days of moist curing.

<sup>2</sup> With respect to the line of continuous brick courses.

<sup>3</sup> Flexural strength computed with neutral axis assumed at the center of the slab; each result the average of three tests.

<sup>4</sup> Plain concrete slabs were taken as standard.

The right panel in figure 5 gives results for slabs tested with the top surface, as cast, in tension, the courses of brick being placed at an angle of 45° to the axis of the loading knife edge. The effect of having the brick in tension was to lower the strength to 62 percent of that of plain concrete. This reduction in strength is also in close agreement with the results obtained in

series A where the strength ratio was 60 percent for 8-inch monolithic brick slabs tested with the brick surface in tension and the brick courses at an angle of 45°.

The data plotted in figure 5 are of interest in giving a comparison of monolithic brick slabs using concrete of base mix proportions with plain concrete slabs of pavement proportions. As will be noted by comparing the first and fourth bars in the left panel of figure 5, the monolithic brick slabs using base mix proportions gave almost identical results with those obtained for the plain concrete slabs of pavement proportions. This, of course, was with the brick surface in compression. However, where the slabs were tested with the brick surface in tension, the monolithic brick slabs using base concrete were very much lower in strength.

Summarizing the data for series B, the results check very closely with those found in series A, where comparable tests were made. In addition, it was found that monolithic brick slabs made with base concrete were equal in strength to vibrated slabs of pavement concrete, when tested with the bottom in tension.

#### SUMMARY

The wheel load of a vehicle on a rigid pavement produces tensile stress in the bottom of the slab when the load is at the interior of the slab at some distance from an edge, or at the edge of the slab at some distance from a corner. Therefore, with respect to resistance to loads at the interior and edges of monolithic brick pavements, the most significant results of these tests were the strengths obtained with the slabs tested with the bottoms in tension.

On the other hand, a concentrated load applied at the corner of a rigid pavement produces tensile stress in the top of the slab and, if rupture occurs it will take place along a line that is roughly at an angle of 45° with the axis of the slab. Thus, if a monolithic brick pavement fails under a corner load, the slab will be ruptured along a line that is at an angle of approximately 45° with the direction of the courses of brick. The test slabs with the brick placed at an angle of 45° with the direction of the applied load were built to simulate this corner condition. When tested with the brick surface in tension they ruptured in the same direction, with respect to the brick courses, as the corner of a monolithic brick pavement would be expected to rupture. Therefore, of the results obtained with the

brick slabs tested with the tops in tension, the most significant, with respect to their application in pavement design, are the strengths of the slabs with the brick placed at an angle of 45°.

With the above discussion in mind, the results of these tests may be summarized as follows, all comparisons being made on the basis of slabs of equal thickness:

1. Monolithic brick slabs, when tested with the concrete in tension, developed flexural strengths ranging from 106 to 119 percent of the strengths of comparable slabs of plain concrete.

2. Monolithic brick slabs of base concrete and plain concrete slabs of pavement concrete, when tested with the concrete in tension, developed approximately the same strengths.

3. In monolithic brick slabs tested with the concrete in tension, the bond between the brick surface and the concrete base apparently was not affected by 100 applications of a load equal to 50 percent of the ultimate load.

4. Monolithic brick slabs with the brick at an angle of 45° with the axis of the slab and tested with the brick surface in tension developed flexural strengths ranging from 60 to 76 percent of the strengths of comparable slabs of plain concrete.

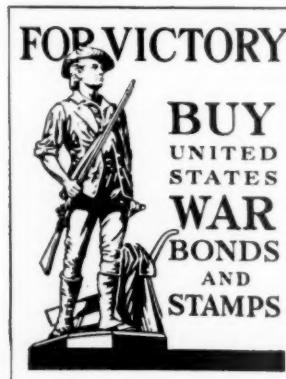
5. Monolithic brick slabs of base concrete, with the brick at an angle of 45° with the axis of the slab and tested with the brick surface in tension, developed about 53 percent of the strength of plain concrete slabs of pavement concrete.

#### HIGHWAY RESEARCH BOARD MEETS IN ST. LOUIS, DECEMBER 2-4, 1942

The Twenty-second Annual Meeting of the Highway Research Board will be held December 2-4, 1942, at the Hotel Statler in St. Louis, Mo.

The customary sessions of the board for the discussion of topics relating to highway finance, economics, design, materials, construction, maintenance, traffic, and soils investigation will be held.

It is expected that time and travel facilities will be greatly conserved by holding this meeting contiguous to that of the American Association of State Highway Officials which will be held in St. Louis the following week.



## MOTOR-FUEL CONSUMPTION-1941

COMPILED FOR CALENDAR YEAR FROM REPORTS OF STATE AUTHORITIES 1/

TABLE G-2, 1941  
ISSUED JUNE 1942

STATE	TAX RATE PER GALLON ON DECEMBER 31	GROSS AMOUNT REPORTED 2/	AMOUNT EXEMPTED FROM PAYMENT OF TAX 3/	GROSS AMOUNT ASSESSED FOR TAXATION	AMOUNT SUBJECT TO REFUND OF ENTIRE TAX	NET AMOUNT TAXED			AMOUNT TAXED AT PREVAILING RATE DURING 1940	INCREASE DURING 1941		STATE			
						TOTAL	AT PREVAILING RATE	AT OTHER RATES							
								PER GALLON	AMOUNT						
ALABAMA	6	310,603	—	310,603	—	310,603	310,603	—	259,915	50,688	19.5	ALABAMA			
ARIZONA	5	130,954	8,060	122,894	12,883	110,011	110,011	—	95,707	14,304	14.9	ARIZONA			
ARKANSAS	6.5	223,634	8,872	214,762	214,762	190,095	190,095	(4/)	162,328	27,767	17.1	ARKANSAS			
CALIFORNIA	3	2,272,732	119,055	2,153,677	175,130	1,978,547	1,978,547	—	1,758,326	220,221	12.5	CALIFORNIA			
COLORADO	4	270,287	14,279	256,008	34,489	221,519	221,519	—	206,742	14,777	7.1	COLORADO			
CONNECTICUT	3	422,586	14,302	408,284	9,735	398,549	398,549	—	361,594	33,955	9.3	CONNECTICUT			
DELAWARE	4	69,122	1,837	67,285	4,197	63,088	63,088	—	58,397	4,691	8.0	DELAWARE			
FLORIDA	7	475,905	49,446	426,459	—	426,459	426,459	—	388,717	37,742	9.7	FLORIDA			
GEORGIA	6	449,335	22,277	427,058	—	427,058	427,058	—	386,847	40,211	10.6	GEORGIA			
IDAHO	5.5	117,232	4,124	113,108	—	102,255	102,255	(5/)	94,565	7,699	8.1	IDAHO			
ILLINOIS	3	1,670,862	135,385	1,535,477	1,535,477	1,410,967	1,410,967	—	1,110,967	124,510	8.8	ILLINOIS			
INDIANA	4	799,091	7,484	791,607	70,074	721,533	721,533	—	641,164	80,369	12.5	INDIANA			
IOWA	3	612,221	—	612,221	93,072	519,149	519,149	—	485,994	33,158	6.8	IOWA			
KANSAS	3	541,494	162,862	378,632	—	378,632	378,632	—	346,770	31,912	9.2	KANSAS			
KENTUCKY	5	352,743	15,527	337,716	—	337,716	337,716	—	295,205	47,011	14.7	KENTUCKY			
LOUISIANA	7	343,599	29,263	314,336	6	304,900	304,900	2	266,057	40,843	15.5	LOUISIANA			
MAINE	4	173,874	1,461	172,413	—	163,942	163,942	1	128,471	14,812	9.9	MAINE			
MARYLAND	4	305,530	10,006	295,524	22,626	332,898	329,903	3	286,636	43,267	15.1	MARYLAND			
MASSACHUSETTS	3	801,957	9,609	792,348	34,277	758,071	758,071	—	713,358	44,713	6.3	MASSACHUSETTS			
MICHIGAN	3	1,392,274	128,012	1,264,262	64,766	1,199,498	1,199,498	1.5	1,082,006	116,466	10.8	MICHIGAN			
MINNESOTA	10/ 4	625,173	33,640	591,533	71,526	520,007	520,007	—	497,556	25,451	4.5	MINNESOTA			
MISSISSIPPI	6	254,877	14,897	239,980	—	239,980	239,980	1	197,570	34,888	17.7	MISSISSIPPI			
MISSOURI	2	776,432	—	776,432	38,710	737,722	737,722	—	659,321	79,401	12.1	MISSOURI			
MONTANA	5	148,262	6,717	141,545	28,296	113,249	113,249	—	104,511	8,798	8.4	MONTANA			
NEBRASKA	5	259,765	11,255	248,510	21	248,489	248,339	1	233,438	14,901	6.4	NEBRASKA			
NEVADA	4	49,018	2,909	46,109	3,108	43,001	43,001	5	40,807	35,717	14.3	NEVADA			
NEW HAMPSHIRE	4	102,395	896	101,499	5,055	96,444	96,444	—	90,822	5,622	6.2	NEW HAMPSHIRE			
NEW JERSEY	3	1,006,334	42,230	966,104	85,920	878,184	878,184	—	822,426	55,758	6.8	NEW JERSEY			
NEW MEXICO	5	122,275	6,986	115,289	12,221	103,068	103,068	—	96,196	8,972	9.4	NEW MEXICO			
NEW YORK	4	2,058,071	90,529	1,967,542	69,061	1,896,481	1,896,481	—	1,836,977	61,304	3.3	NEW YORK			
NORTH CAROLINA	6	526,341	—	526,341	526,341	511,064	511,064	1	447,548	73,516	16.0	NORTH CAROLINA			
NORTH DAKOTA	4	160,615	70,770	90,045	—	90,045	90,045	—	85,760	4,205	5.0	NORTH DAKOTA			
OHIO 15/	4	1,638,193	90,737	1,548,176	18,675	1,529,501	1,446,636	1	1,320,885	125,747	9.5	OHIO			
OKLAHOMA	16/ 5.5	473,597	78,555	395,042	395,042	375,755	375,755	—	326,390	35,736	15.6	OKLAHOMA			
OREGON	5	306,484	9,348	297,136	31,167	265,969	264,132	1	228,184	35,736	15.6	OREGON			
PENNSYLVANIA	4	1,704,947	11,201	1,693,746	—	1,693,746	1,693,746	—	1,575,078	118,668	7.5	PENNSYLVANIA			
RHODE ISLAND	3	147,280	3,605	143,675	1,445	142,230	142,230	—	131,558	10,672	8.1	RHODE ISLAND			
SOUTH CAROLINA	6	275,514	—	275,514	9,416	266,098	266,098	—	229,398	36,700	16.0	SOUTH CAROLINA			
SOUTH DAKOTA	4	156,036	2,681	153,355	33,804	119,551	119,551	—	112,659	5,892	5.2	SOUTH DAKOTA			
TENNESSEE	7	266,300	26,316	269,316	4,955	355,029	355,029	—	204,042	50,987	16.8	TENNESSEE			
TEXAS	4	1,604,602	101,499	1,563,103	221,435	1,341,668	1,341,668	—	1,195,696	145,978	12.2	TEXAS			
UTAH	4	118,062	4,356	113,706	—	113,706	102,425	—	112,281	11,281	11.0	UTAH			
VERMONT	4	75,189	944	74,245	—	74,245	69,986	—	64,319	6,622	10.2	VERMONT			
VIRGINIA	5	503,281	—	503,281	31,884	471,797	471,797	3	392,286	78,633	20.0	VIRGINIA			
WASHINGTON	5	450,676	28,855	421,821	28,753	399,068	399,068	—	347,872	45,196	13.0	WASHINGTON			
WEST VIRGINIA	4	242,167	—	242,167	8,227	233,940	233,940	—	214,983	18,957	8.8	WEST VIRGINIA			
WISCONSIN	4	636,760	18,844	617,916	42,334	575,582	575,582	—	529,723	45,849	8.7	WISCONSIN			
WYOMING	4	79,840	2,531	77,309	77,309	77,309	77,309	—	60,999	8,310	12.0	WYOMING			
DISTRICT OF COLUMBIA	20/ 2	191,920	9,758	182,162	3,578	178,584	178,584	—	160,958	17,630	11.0	DISTRICT OF COLUMBIA			
TOTAL	21/ 3.99	26,937,361	1,276,535	25,660,826	1,405,829	24,254,997	24,086,922	—	168,075	21,913,441	2,173,481	9.9	TOTAL		

1/ AN ANALYSIS OF MOTOR-FUEL USAGE WILL BE GIVEN IN TABLE G-21, TO BE PUBLISHED LATER.

2/ EXPORT SALES AND OTHER AMOUNTS NOT REPRESENTING CONSUMPTION IN STATE HAVE BEEN ELIMINATED AS FAR AS POSSIBLE. IN CASES WHERE STATES FAILED TO REPORT AMOUNTS EXEMPTED FROM TAXATION, THE GROSS AMOUNT TAXED IS SHOWN IN THIS COLUMN.

3/ INCLUDES ALLOWANCES FOR EVAPORATION AND OTHER LOSSES, FEDERAL USE, OTHER PUBLIC USE, AND NONHIGHWAY USE, WHERE INITIAL EXEMPTIONS RATHER THAN REFUNDS ARE MADE.

4/ WITHIN 300 FEET OF BORDER, TAX IS REDUCED TO THAT OF ADJACENT STATE. GALLONS TAXED AT 2 CENTS, 6,731,000; AT 4 CENTS, 10,019,000; AT 5.5 CENTS, 7,917,000.

5/ AVIATION FUEL TAXED AT 2.5 CENTS, 607,000 GALLONS; MOTOR FUEL TAXED AT 0.1 CENTS (5 CENTS REFUNDED ON NONHIGHWAY USE), 10,246,000 GALLONS.

6/ REPRESENTS EVAPORATION OR LOSS ALLOWANCE UNDER 5-CENT TAX NOT ALLOWED UNDER ADDITIONAL 2-CENT TAX, WHICH IS ADMINISTERED UNDER A SEPARATE LAW.

7/ THREE CENTS PER GALLON REFUNDED ON NONHIGHWAY USES.

8/ ONE CENT PER GALLON REFUNDED ON MOTOR FUEL USED IN VEHICLES LICENSED TO OPERATE EXCLUSIVELY IN CITIES.

9/ ONE AND ONE-HALF CENTS PER GALLON REFUNDED ON MOTOR FUEL USED IN INTERSTATE AVIATION.

10/ RATE CHANGED FROM 3 CENTS TO 4 CENTS MAY 1.

11/ TAXED AT 3 CENTS, 142,040,000 GALLONS; AT 4 CENTS, 377,963,000 GALLONS.

12/ FIVE CENTS PER GALLON REFUNDED ON NONHIGHWAY USES.

13/ FOUR CENTS PER GALLON REFUNDED ON MOTOR FUEL USED IN FLYING INSTRUCTION.

14/ DIESEL FUEL AND BUTANE.

15/ AMOUNTS GIVEN DO NOT INCLUDE 6,317,000 GALLONS OF LIQUID FUEL (KEROSENE, FUEL OIL, ETC.) TAXED AT 1 CENT PER GALLON BUT NOT SUBJECT TO THE 3-CENT TAX ON MOTOR-VEHICLE FUEL.

16/ RATE CHANGED FROM 4 CENTS TO 5.5 CENTS JUNE 1.

17/ TAXED AT 4 CENTS, 163,980,000 GALLONS; AT 5.5 CENTS, 231,062,000 GALLONS.

18/ FOUR CENTS PER GALLON REFUNDED ON MOTOR FUEL USED IN AVIATION.

19/ TWO CENTS PER GALLON REFUNDED ON MOTOR FUEL USED IN INTRASTATE AVIATION.

20/ RATE CHANGED FROM 2 CENTS TO 3 CENTS JANUARY 1, 1942.

21/ WEIGHTED AVERAGE RATE.

## STATE MOTOR-FUEL TAX RECEIPTS - 1941

COMPILED FOR CALENDAR YEAR FROM REPORTS OF STATE AUTHORITIES

TABLE G-1, 1941  
ISSUED JUNE 1942

STATE	TAX RATE PER GALLON ON DECEMBER 31	RECEIPTS FROM TAXATION OF MOTOR FUEL					OTHER RECEIPTS IN CONNECTION WITH MOTOR-FUEL TAX <sup>2/</sup>					NET TOTAL RECEIPTS	LESS TAX ON AVIATION GASOLINE	ADJUSTED NET TOTAL RECEIPTS	STATE
		GROSS TAX COLLECTIONS	DEDUCTIONS BY DISTRIBUTORS FOR EXPENSES <sup>1/</sup>	GROSS RECEIPTS BY STATE	REFUNDS PAID	NET RECEIPTS BY STATE	DISTRIBUTORS AND DEALERS LICENSES	INSPECTION FEES <sup>2/</sup>	FINES AND PENALTIES	MISCELLANEOUS RECEIPTS	TOTAL				
ALABAMA	6	18,323	-	18,323	-	18,323	-	78	-	*	78	18,401	-	18,401	ALABAMA
ARIZONA	5	6,148	-	6,148	787	5,361	*	-	1	-	1	5,362	-	5,362	ARIZONA
ARKANSAS	6.5	13,100	-	13,100	-	13,100	-	119	-	-	119	13,219	-	13,219	ARKANSAS
CALIFORNIA	3	63,330	-	63,330	5,254	58,076	17	-	-	1	18	58,096	-	58,096	CALIFORNIA
COLORADO	4	10,195	-	10,195	1,362	8,833	-	-	-	-	-	8,833	-	8,833	COLORADO
CONNECTICUT	3	12,180	122	12,058	300	11,758	45	-	1	-	46	11,804	-	11,804	CONNECTICUT
DELAWARE	4	2,665	-	2,665	164	2,501	3	-	-	-	3	2,504	-	2,504	DELAWARE
FLORIDA	7	29,832	-	29,832	-	29,832	41	524	-	-	565	30,397	-	30,397	FLORIDA
GEORGIA	6	26,311	262	26,049	-	26,049	37	-	-	-	37	26,086	-	26,086	GEORGIA
IDAHO	5.1	5,722	-	5,722	491	5,231	1	-	-	*	1	5,232	14	5,218	IDAHO
ILLINOIS	3	49,656	993	48,663	3,901	44,762	-	498	1	-	499	45,261	-	45,261	ILLINOIS
INDIANA	4	31,270	-	31,270	2,752	28,518	*	700	24	2	736	29,244	-	29,244	INDIANA
LOUISIANA	3	18,199	-	18,199	2,861	15,338	62	-	-	-	62	15,400	-	15,400	LOUISIANA
KANSAS	3	11,234	-	11,234	-	11,234	15	118	-	22	155	11,389	-	11,389	KANSAS
KENTUCKY	5	16,336	163	16,175	-	16,175	-	-	5	-	5	16,180	-	16,180	KENTUCKY
LAWAVERA	7	21,425	-	21,425	-	21,425	-	107	30	-	137	21,562	-	21,562	LAWAVERA
MAINE	4	6,918	-	6,918	261	6,657	-	-	*	*	*	6,657	3	6,658	MAINE
MARYLAND	4	14,000	-	14,000	930	13,070	-	-	-	-	13,070	-	13,070	MARYLAND	
MASSACHUSETTS	3	23,576	-	23,576	1,088	22,488	48	-	*	-	48	22,536	-	22,536	MASSACHUSETTS
MICHIGAN	3	37,586	-	37,586	2,001	35,585	5	-	2	-	7	35,592	88	35,592	MICHIGAN
MINNESOTA	3/ 4	21,570	-	21,570	2,700	18,870	*	182	3	*	185	19,055	-	19,055	MINNESOTA
MISSISSIPPI <sup>5/</sup>	6	14,126	-	14,126	373	13,753	-	-	-	-	13,753	-	13,753	MISSISSIPPI <sup>5/</sup>	
MISSOURI	2	15,252	-	15,252	975	14,277	-	-	165	9	174	14,451	-	14,451	MISSOURI
MONTANA	5	6,878	-	6,878	1,411	5,467	-	4	-	-	4	5,471	-	5,471	MONTANA
NEBRASKA	5	12,711	94	12,617	349	12,268	11	128	-	34	173	12,441	84	12,357	NEBRASKA
NEVADA	4	1,682	34	1,682	125	1,683	*	24	1	-	25	1,708	-	1,708	NEVADA
NEW HAMPSHIRE	4	3,913	-	3,913	198	3,715	-	-	1	-	1	3,716	-	3,716	NEW HAMPSHIRE
NEW JERSEY	3	26,814	-	26,814	2,599	26,221	69	-	2	14	85	26,306	-	26,306	NEW JERSEY
NEW MEXICO	5	5,695	-	5,695	619	5,076	26	-	-	-	26	5,102	-	5,102	NEW MEXICO
NEW YORK	4	78,081	780	77,261	2,803	74,458	62	-	*	-	62	74,520	-	74,520	NEW YORK
NORTH CAROLINA	6	31,417	-	31,417	768	30,649	-	1,271	-	11	1,282	31,931	-	31,931	NORTH CAROLINA
NORTH DAKOTA	4	3,569	54	3,515	-	3,515	1	83	-	26	110	3,625	-	3,625	NORTH DAKOTA
OHIO	4	61,274	-	61,274	3,161	58,113	-	-	-	-	-	58,113	-	58,113	OHIO
OKLAHOMA	2/ 5.5	19,181	478	18,663	2	18,661	-	315	-	-	315	18,976	-	18,976	OKLAHOMA
OREGON	4	14,702	-	14,702	1,673	13,029	-	-	*	-	13,029	-	13,029	OREGON	
PENNSYLVANIA	4	65,627	760	64,867	-	64,867	-	-	5	-	5	64,872	-	64,872	PENNSYLVANIA
RHODE ISLAND	3	4,627	-	4,627	355	4,272	3	-	-	-	3	4,275	-	4,275	RHODE ISLAND
SOUTH CAROLINA	6	16,356	-	16,356	539	15,817	-	330	-	-	330	16,147	87	16,061	SOUTH CAROLINA
SOUTH DAKOTA	4	6,112	215	5,897	1,338	4,559	-	79	-	-	79	4,638	21	4,617	SOUTH DAKOTA
TENNESSEE	7	24,505	-	24,505	315	24,190	-	1,386	-	-	69	25,645	246	25,399	TENNESSEE
TEXAS	4	62,972	633	62,339	9,598	52,747	-	-	-	21	21	52,768	-	52,768	TEXAS
UTAH	4	4,500	68	4,432	-	4,432	1	-	1	*	2	4,434	64	4,434	UTAH
VERMONT	4	2,930	-	2,930	-	2,930	-	-	*	-	2,930	-	2,930	VERMONT	
VIRGINIA	5	26,501	-	26,501	1,585	22,916	1	-	2	*	3	22,919	23	22,936	VIRGINIA
WASHINGTON	5	20,884	104	20,780	1,419	19,321	3	-	-	10	13	19,334	-	19,334	WASHINGTON
WEST VIRGINIA	5	11,956	-	11,956	403	11,553	6	-	-	6	6	11,559	-	11,559	WEST VIRGINIA
WISCONSIN	4	24,432	-	24,432	1,695	22,737	-	189	-	-	189	22,986	-	22,986	WISCONSIN
WYOMING	4	3,065	-	3,065	-	3,065	2	-	-	-	2	3,067	53	3,014	WYOMING
DISTRICT OF COLUMBIA	2/ 2	3,550	-	3,550	71	3,479	*	-	-	*	3	3,479	-	3,479	DISTRICT OF COLUMBIA
TOTAL	2/ 3.99	1,012,930	4,760	1,008,170	57,214	950,956	459	6,300	88	210	7,057	958,013	701	957,312	TOTAL

<sup>1/</sup> THE STATES FOR WHICH AMOUNTS ARE SHOWN MAKE ALLOWANCES TO DISTRIBUTORS FOR EXPENSE OF COLLECTING THE TAX. IN KENTUCKY, SOUTH DAKOTA, UTAH, AND WASHINGTON ALLOWANCES OF 2 1/4, 4, 3, AND 1 PERCENT, RESPECTIVELY, OF THE TAX OTHERWISE DUE ARE MADE IN CONSIDERATION OF BOTH EXPENSE OF COLLECTION AND GALLONAGE LOSSES IN HANDLING. IN THESE STATES THE ALLOWANCES FOR EXPENSES ONLY HAVE BEEN ESTIMATED AS 1, 3 1/2, 1 1/2, AND 1/2 PERCENT, RESPECTIVELY.

<sup>2/</sup> STARS INDICATE AMOUNTS LESS THAN \$500.

<sup>3/</sup> FEES FOR INSPECTION OF MOTOR-VEHICLE FUEL. WHEREVER POSSIBLE, FEES FOR INSPECTION OF KEROSENE AND OTHER NON-MOTOR-VEHICLE FUELS HAVE BEEN ELIMINATED.

<sup>4/</sup> RATE CHANGED FROM 3 CENTS TO 4 CENTS MAY 1.

<sup>5/</sup> SPECIAL COUNTY TAXES OF 3 CENTS PER GALLON IN HANCOCK COUNTY AND 2 CENTS PER GALLON IN HARRISON AND JACKSON COUNTIES, AMOUNTING TO \$306,000 IN 1941, ARE IMPOSED FOR SEAWALL PROTECTION AND ARE NOT INCLUDED IN THIS TABLE.

<sup>6/</sup> OHIO IMPOSES A 3-CENT TAX ON MOTOR-VEHICLE FUEL AND A 1-CENT TAX ON ALL LIQUID FUELS. THE RECEIPTS FROM THE 1-CENT TAX APPLICABLE TO NON-MOTOR-VEHICLE FUELS (KEROSENE, FUEL OIL, ETC.) WERE \$627,000. THESE RECEIPTS HAVE BEEN ELIMINATED FROM THE TOTAL GIVEN, WHICH REPRESENTS A 4-CENT TAX ON MOTOR-VEHICLE FUEL.

<sup>7/</sup> RATE CHANGED FROM 4 CENTS TO 5.5 CENTS JUNE 1.

<sup>8/</sup> THIS RATE NO LONGER IN EFFECT; RATE CHANGED FROM 2 CENTS TO 3 CENTS JANUARY 1, 1942.

<sup>9/</sup> WEIGHTED AVERAGE RATE.



## STATE MOTOR-VEHICLE RECEIPTS - 1941

COMPILED FOR CALENDAR YEAR FROM REPORTS OF STATE AUTHORITIES<sup>1/</sup>

TABLE No. 2, 1941

(ISSUED JUNE 1942)

STATE	REGISTRATION FEES <sup>2/</sup>										MISCELLANEOUS RECEIPTS										STATE														
	MOTOR VEHICLES					OTHER VEHICLES					DEALERS' LICENSES					OPERATORS' PERMITS																			
	PASSENGER VEHICLES		AUTOMOBILES		BUSES <sup>3/</sup>	TOTAL		TRAILERS		MOTOR-CYCLES	TOTAL		DEALERS' LICENSES		TITLE FEES		SPECIAL TITLING FEES <sup>4/</sup>		TRANSFER OF REGISTRATION FEES <sup>5/</sup>		FINES AND PENALTIES		ESTIMATED SERVICE CHARGES		OTHER RECEIPTS LESS LOCAL, UNCLASSIFIED REFUNDS										
ALABAMA	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000	1,000								
ARIZONA	4,100	4,100	131	4,231	1,317	5,548	74	6	5,628	3	601	9	601	9	601	9	182	7	17	17	17	17	17	17	17	17	17	17	17	17	17	17			
ARKANSAS	3,635	4,00	25	4,224	1,335	3,359	199	1	3,559	101	254	119	254	119	254	119	4	15	4	15	4	15	4	15	4	15	4	15	4	15	4	15			
CALIFORNIA <sup>6/</sup>	2,927	22,166	(2/)	22,166	26,050	1,760	91	29,177	1,497	2,275	46	2,233	35	168	197	179	100	57	100	315	2,903	2,903	2,903	2,903	2,903	2,903	2,903	2,903	2,903	2,903	2,903	2,903	2,903	2,903	2,903
COLORADO	2,953	8,778	-	8,778	3,575	1,237	5,312	27	5,346	115	2,090	100	2,090	100	2,090	100	179	178	179	178	179	179	179	179	179	179	179	179	179	179	179	179			
CONNECTICUT	8,249	3,417	158	739	1,739	1,320	50	1	1,171	9	114	68	114	68	114	68	52	36	52	36	52	36	52	36	52	36	52	36	52	36	52	36			
DELAWARE	1,456	1,456	739	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456	1,456						
FLORIDA	2,463	2,463	266	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463	2,463					
GEORGIA	2,955	1,298	160	1,298	1,298	505	1,963	205	2,171	17	725	-	725	-	725	-	11	-	11	-	11	-	11	-	11	-	11	-	11	-	11				
IDAHO	1,542	673	10	1,542	1,542	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683	1,683					
ILLINOIS	26,567	16,415	16,415	26,567	26,567	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415	16,415						
INDIANA	11,628	7,142	156	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142	7,142						
KANSAS	10,231	9,216	216	9,216	9,216	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019	1,019					
KENTUCKY	6,793	6,793	2,023	6,793	6,793	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023	2,023						
LOUISIANA	3,426	10,187	118	3,426	3,426	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236	1,236					
MAINE	4,100	2,980	21	2,980	2,980	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206					
MARYLAND	7,294	3,599	221	3,599	3,599	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206					
MASSACHUSETTS	2,925	1,622	163	2,925	2,925	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206					
MICHIGAN	27,165	12,017	12,017	27,165	27,165	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017	12,017					
MINNESOTA <sup>10/</sup>	19,230	16,724	222	16,724	16,724	3,654	3,654	3,654	3,654	18,597	680	13	18,597	680	13	18,597	680	13	18,597	680	13	18,597	680	13	18,597	680	13	18,597	680	13					
MISSOURI	3,203	2,713	231	2,713	2,713	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206					
MISSOURI <sup>11/</sup>	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297	1,297					
NEBRASKA	3,195	1,960	27	1,960	1,960	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206					
NEVADA	4,071	1,911	1	1,911	1,911	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206					
NEW HAMPSHIRE	28,249	11,774	316	11,774	11,774	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206					
NEW JERSEY	2,912	1,718	68	1,718	1,718	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206					
NEW MEXICO	5,977	36,077	100	36,077	36,077	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207	1,207					
NEW YORK	53,977	50,622	17,317	17,317	17,317	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677	9,677					
North Dakota	57,777	17,421	248	17,421	17,421	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206					
OKLAHOMA	7,086	4,916	43	4,916	4,916	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206	1,206					
PENNSYLVANIA	4,416	2,173	213	2																															

## STATUS OF FEDERAL-AID HIGHWAY PROJECTS

AS OF JULY 31, 1942

STATE	COMPLETED DURING CURRENT FISCAL YEAR			UNDER CONSTRUCTION			APPROVED FOR CONSTRUCTION			BALANCE OF FUNDS AVAILABLE FOR GRAMMED PROJECTS
	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	
Alabama	\$4,261,349	\$2,315,384	115.2	\$195,900	\$99,450	0.3	\$2,565,784			
Arizona	665,314	528,164	28.2	487,374	301,290	4.8	1,515,952			
Arkansas	1,275,742	635,815	59.9	810,614	589,082	16.9	1,172,187			
California	4,473,982	3,124,165	42.6	2,610,776	1,856,813	75.5	2,126,638			
Colorado	3,541,754	2,270,652	155.4	938,023	529,525	30.9	1,954,086			
Connecticut	2,254,409	1,090,642	27.5	1,185,422	774,473	6.7	1,24,612			
Delaware	4,28,810	210,460	9.0	268,040	134,020	8.4	1,51,348			
Florida	45,075	3,419,614	2,017,687	52.7	150,000	112,500	101.5	2,909,335		
Georgia	193,521	8,062,690	4,254,197	3,030,647	1,581,005	101.5	6,189,049			
Idaho	307,561	1,109,663	812,171	4,251,377	321,944	10.6	1,529,251			
Illinois	739,150	1,109,663	4,078,892	114.5	2,524,528	1,602,596	23.1	5,29,261		
Indiana	805,800	1,022,900	15.5	6,425,889	3,129,737	89.7	1,20,420			
Iowa	118,768	196,300	21.3	3,644,785	1,521,195	92.7	123,116	27,200	4.6	2,90,557
Kansas	346,156	181,060	19.0	5,964,404	3,264,041	240.2	659,053	339,117	34.9	4,50,209
Kentucky	283,274	141,637	6.2	5,089,499	2,577,107	87.0	3,263,364	1,948,234	31.5	574,514
Louisiana	15,648	11,736	1,159,867	798,261	22.4	3,755,006	2,111,722	67.5	3,42,906	
Maine	155,460	77,140	2.9	1,789,292	917,737	19.2	78,610	39,305	1	1,006,879
Maryland	144,074	221,658	5.5	2,233,181	1,102,489	12.0	810,000	573,625	4.3	780,082
Massachusetts	25,000	21,000	4.4	1,493,894	821,486	6.2	1,170,360	561,593	8.4	3,802,058
Michigan	934,753	576,391	44.1	3,526,216	2,343,158	73.7	1,477,310	1,006,775	61.0	2,340,674
Minnesota	44,1	8,463,111	2,923,908	4,597,110	347,110	347.1	499,190	246,595	2,631,052	
Mississippi	699,496	349,728	14.3	9,353,372	5,999,469	167.4	331,300	248,715	5.3	2,077,457
Missouri	680,704	109,067	18.3	3,976,532	2,018,054	125.3	238,580	158,250	25.7	4,088,704
Montana	687,804	358,622	57.6	2,260,350	1,143,999	125.5	1,167,560	874,778	76.6	3,644,730
Nebraska	97,818	85,002	6.4	1,479,26	1,324,000	40.8	544,179	486,949	13.9	4,23,339
New Hampshire		1,682,125	1,131,880	19.4						683,756
New Jersey		65,262	42,631							2,381,651
New Mexico	55,592	35,583	11.0	726,233	566,823	26.9	818,715	671,755	56.5	1,977,719
New York	804,019	514,788	9.3	7,952,443	5,067,685	77.0	429,000	193,500	6.4	5,204,933
North Carolina	868,374	1,055,347	24.2	1,122,567	599,147	51.5	852,016	445,552	10.0	3,484,755
Ohio	338,180	215,387	41.9	2,922,673	1,757,395	190.0	2,851,800	1,636,982	181.8	3,010,930
Oklahoma	2,757,360	1,175,944	16.2	2,047,719	7,049,032	70.2	3,527,610	1,938,966	27.7	757,003
Oregon	112,243	51,000	10.4	2,717,873	1,521,582	57.2	1,787,660	1,090,044	45.0	5,756,946
Pennsylvania	365,287	182,489	4.3	3,755,534	2,128,876	75.7	397,962	318,310	26.2	920,823
Rhode Island	14,721	7,312	1,032,274	4,527,019	61.0	3,036,620	2,235,739	13.6	2,667,919	
South Carolina										
South Dakota	517,100	404,170	57.5	3,775,133	2,491,345	544.0	615,240	365,050	70.4	2,851,421
Tennessee	1,351,060	163,154	1.5	4,250,726	2,496,580	95.2				
Texas	715,100	51.6	7,197,714	3,399,98	189.4	1,330,145	651,130	40.9	8,672,202	
Utah	577,845	434,655	14.8	1,132,810	901,211	26.8	874,733	476,706	23.2	766,837
Vermont	237,535	107,173	2.8	955,791	608,167	18.0	36,318	18,453	0.3	356,713
Virginia	305,788	155,839	8.8	2,663,666	1,625,853	42.2	35,190	21,455	4.4	2,751,658
Washington	40,957	27,150	8.4	2,639,710	1,473,993	30.1	211,457	124,500	3.4	1,835,522
West Virginia	1,233,500	208,401	4.4	2,485,655	1,502,080	20.4	884,959	488,979	8.8	1,099,368
Wisconsin	1,289,778	791,990	51.9	5,010,079	3,211,467	117.5	252,531	251,150	10.4	3,399,384
Wyoming	491,888	353,584	27.4	1,358,229	1,040,201	104.7	140,715	115,749	12.1	1,099,972
District of Columbia	3,444	3,444	3.1	1,336,529	206,688	11.8	1,003,052	593,156	3.6	285,520
Hawaii	81,880	2,425,620	1,199,666	1,199,666	17.3	476,174	456,702	4.7	1,366,291	
Puerto Rico	166,025					37,325	27,994	2.2	527,567	
TOTALS	18,452,645	10,699,930	600.8	177,026,119	102,993,305	4,421.4	50,935,127	31,308,034	1,227.6	121,639,890

## STATUS OF FEDERAL-AID SECONDARY OR FEEDER ROAD PROJECTS

AS OF JULY 31, 1942

STATE	COMPLETED DURING CURRENT FISCAL YEAR			UNDER CONSTRUCTION			APPROVED FOR CONSTRUCTION			BALANCE OF FUNDING AVAILABLE FOR PROGRAMMED PROJECTS
	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	Estimated Total Cost	Federal Aid	Miles	
Alabama	\$63,345	\$46,046	2.9	\$562,252	\$300,210	10.9	\$23,011	\$11,050	49.650	\$590,870
Arizona	4,995	4,995		142,693	103,688	6.7	69,383	49.650	49.777	494,712
California				662,780	329,780	32.4	135,656	67,416	6.0	180,507
Colorado				703,507	555,721	8.2	18,050	9,500		
Connecticut				131,809	78,621	3.2	152,387	35,323	5.0	197,935
Delaware				275,575	120,602	4.8				194,440
Florida				8,040	4,020		102,873	37,618	3.9	291,159
Georgia				138,120	103,530	6.0				399,166
Idaho				1,326,307	775,503	82.8	604,762	302,381	36.6	938,471
Illinois				391,287	259,816	12.2	38,149	23,587	2.8	183,196
Indiana	40,000	20,000	2.2	1,261,384	631,692	63.2	219,000	109,500	3.7	771,146
Iowa	80,000	40,000	5.3	1,269,466	599,282	48.8				927,827
Kansas	109,341	51,500	20.6	543,562	242,510	79.1	32,056	15,050	8.8	985,167
Kentucky	26,060	17,170	1.7	1,803,040	904,857	97.9	177,337	88,669	23.4	1,039,534
Louisiana	140,734	22,951	4.5	1,041,831	378,281	29.9	35,126	6,600	1.1	319,467
Maine	64,773	32,385	3.8	15,460	7,130		674,013	267,134	15.3	533,140
Maryland				116,796	58,398	4.1	39,383	19,614	0.5	142,227
Massachusetts	281,682	128,841	3.4	226,410	113,205	1.8				345,564
Michigan	51,850	25,900	0.1	462,552	264,842	6.7				522,088
Minnesota	34,062	11,031		672,998	336,409	29.4	219,670	109,835	19.6	655,791
Mississippi				1,021,033	521,505	98.0	298,260	148,730		560,396
Missouri	162,104	81,052	12.5	1,504,188	714,285	99.0				388,361
Montana	32,842	22,609	7.3	560,311	271,114	57.7	186,244	63,193	32.4	1,030,471
Nebraska	54,055	29,503	10.8	252,900	148,113	23.7	13,569	7,715	4.5	904,396
Nevada				131,127	68,477	1.9				695,443
New Hampshire				88,578	56,842	4.6	107,839	90,444	11.9	144,428
New Jersey				261,995	164,766	3.6				874,696
New Mexico				163,825	100,315	7.6				561,260
New York				262,325	169,211	19.5				250,178
North Carolina	240,226	120,230	17.3	1,431,250	756,263	15.9	69,820	20,000	5.0	1,076,253
North Dakota				394,121	214,458	20.8	808,050	793,860		638,766
Ohio				7,382	7,382		236,950	214,475	5.0	755,112
Oklahoma				901,610	498,775	14.5				1,392,467
Oregon				120,588	63,680	4.0	1,215,766	641,924	71.9	860,552
Pennsylvania				286,801	163,737	16.8	30,482	18,000	1.1	333,502
Rhode Island				292,174	146,052	5.5	198,147	97,051	2.5	724,357
South Carolina				78,192	42,846	0.8	33,205	16,622	.2	80,038
South Dakota				24,944	18,355					394,792
Tennessee				9,056	9,056		1,143,430	1,047,600	114.5	751,118
Texas				84,000	453,000	34.8	158,042	79,021	4.6	749,292
Utah	55,035	27,543	2.5	360,490	177,895	26.5				2,021,914
Vermont				99,045	73,875	1.5				305,370
Virginia	322,312	142,308	7.7	222,512	111,328	8.9				933,380
Washington				49,604	25,572	1.7				679,170
West Virginia	175,950	87,975	5.3	337,818	135,362	7.6	786,758	409,402	64.8	1,029
Wisconsin	61,830	.3		115,470	51,122	1.1				511,059
Wyoming	129,993	464,690		1,157,387	528,605	42.4				591,141
District of Columbia				204,982	31.7					214,778
Hawaii							107,026	52,000	.9	111,600
Puerto Rico							49,687	49,687	.4	285,187
TOTALS	3,076,120	1,485,539	130.3	23,335,199	12,099,662	1,053.1	7,900,731	4,506,451	490.5	27,935,338

# *PUBLICATIONS of the PUBLIC ROADS ADMINISTRATION*

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Any of the following publications may be purchased from the Superintendent of Documents, Government Printing Office, Washington, D. C. As his office is not connected with the Agency and as the Agency does not sell publications, please send no remittance to the Federal Works Agency.

## *ANNUAL REPORTS*

Report of the Chief of the Bureau of Public Roads, 1931. 10 cents.  
Report of the Chief of the Bureau of Public Roads, 1932. 5 cents.  
Report of the Chief of the Bureau of Public Roads, 1933. 5 cents.  
Report of the Chief of the Bureau of Public Roads, 1934. 10 cents.  
Report of the Chief of the Bureau of Public Roads, 1935. 5 cents.  
Report of the Chief of the Bureau of Public Roads, 1936. 10 cents.  
Report of the Chief of the Bureau of Public Roads, 1937. 10 cents.  
Report of the Chief of the Bureau of Public Roads, 1938. 10 cents.  
Report of the Chief of the Bureau of Public Roads, 1939. 10 cents.  
Work of the Public Roads Administration, 1940, 10 cents.  
Work of the Public Roads Administration, 1941, 15 cents.

## *HOUSE DOCUMENT NO. 462*

Part 1 . . . Nonuniformity of State Motor-Vehicle Traffic Laws. 15 cents.  
Part 2 . . . Skilled Investigation at the Scene of the Accident Needed to Develop Causes. 10 cents.  
Part 3 . . . Inadequacy of State Motor-Vehicle Accident Reporting. 10 cents.  
Part 4 . . . Official Inspection of Vehicles. 10 cents.  
Part 5 . . . Case Histories of Fatal Highway Accidents. 10 cents.  
Part 6 . . . The Accident-Prone Driver. 10 cents.

## *MISCELLANEOUS PUBLICATIONS*

No. 76MP . . . The Results of Physical Tests of Road-Building Rock. 25 cents.  
No. 191MP . . . Roadside Improvement. 10 cents.  
No. 272MP . . . Construction of Private Driveways. 10 cents.  
No. 279MP . . . Bibliography on Highway Lighting. 5 cents.  
Highway Accidents. 10 cents.  
The Taxation of Motor Vehicles in 1932. 35 cents.  
Guides to Traffic Safety. 10 cents.  
An Economic and Statistical Analysis of Highway-Construction Expenditures. 15 cents.  
Highway Bond Calculations. 10 cents.  
Transition Curves for Highways. 60 cents.  
Highways of History. 25 cents.  
Specifications for Construction of Roads and Bridges in National Forests and National Parks. 1 dollar.

## *DEPARTMENT BULLETINS*

No. 1279D . . . Rural Highway Mileage, Income, and Expenditures, 1921 and 1922. 15 cents.  
No. 1486D . . . Highway Bridge Location. 15 cents.

## *TECHNICAL BULLETINS*

No. 55T . . . Highway Bridge Surveys. 20 cents.  
No. 265T . . . Electrical Equipment on Movable Bridges. 35 cents.

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Single copies of the following publications may be obtained from the Public Roads Administration upon request. They cannot be purchased from the Superintendent of Documents.

## *MISCELLANEOUS PUBLICATIONS*

No. 296MP . . . Bibliography on Highway Safety.  
House Document No. 272 . . . Toll Roads and Free Roads. Indexes to PUBLIC ROADS, volumes 6-8 and 10-21, inclusive.

## *SEPARATE REPRINT FROM THE YEARBOOK*

No. 1036Y . . . Road Work on Farm Outlets Needs Skill and Right Equipment.

## *REPORTS IN COOPERATION WITH UNIVERSITY OF ILLINOIS*

No. 303. . . Solutions for Certain Rectangular Slabs Continuous Over Flexible Support.  
No. 304. . . A Distribution Procedure for the Analysis of Slabs Continuous Over Flexible Beams.  
No. 313. . . Tests of Plaster-Model Slabs Subjected to Concentrated Loads.  
No. 314. . . Tests of Reinforced Concrete Slabs Subjected to Concentrated Loads.  
No. 315. . . Moments in Simple Span Bridge Slabs With Stiffened Edges.

## *UNIFORM VEHICLE CODE*

Act I.—Uniform Motor Vehicle Administration, Registration, Certificate of Title, and Antitheft Act.  
Act II.—Uniform Motor Vehicle Operators' and Chauffeurs' License Act.  
Act III.—Uniform Motor Vehicle Civil Liability Act.  
Act IV.—Uniform Motor Vehicle Safety Responsibility Act.  
Act V.—Uniform Act Regulating Traffic on Highways.  
Model Traffic Ordinances.

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A complete list of the publications of the Public Roads Administration, classified according to subject and including the more important articles in PUBLIC ROADS, may be obtained upon request addressed to Public Roads Administration, Willard Bldg., Washington, D. C.

## STATUS OF FEDERAL-AID GRADE CROSSING PROJECTS

AS OF JULY 31, 1942